

Desktop Natural Hazards Risk Assessment Report

Version B

**eliot
sinclair**

**Pegasus Māketē, 1250 Main North Road,
Woodend**

Prepared for Dexin Investments Ltd
503498


Desktop Natural Hazards Risk Assessment Report

Pegasus Māketē, 1250 Main North Road,
Woodend

Prepared for Dexin Investments Ltd
503498

Quality Control Certificate

Eliot Sinclair & Partners Limited
eliotsinclair.co.nz

Action	Name	Signature	Date
Prepared by:	Peter Ngenang Geotechnical Engineer NZDE Civil MEngNZ		04 March 2024
Reviewed by:	Andrei Cotiga Geotechnical Engineer BE (Hons) Civil ME CPEng		04 March 2024
Directed and approved for release by:	Warren Haynes Surveyor Director BSurv FS+SNZ RPSurv		04 March 2024
Status:	Version B		
Release date:	04 March 2024		
Distributed to:	Dexin Investments Ltd		

Version History

Status	Description	Author	Release Date
A	First issue of document	P. Ngenang	20 December 2023
B	Second issue of document	P. Ngenang	04 March 2024

Executive Summary

Eliot Sinclair & Partners Ltd was engaged by Dexin Investments Ltd to undertake a calibrated desktop assessment to support rezoning of the land at Pegasus Māketē - 1250 Main North Road, Woodend ('the site') and to provide preliminary geotechnical foundation recommendations.

Eliot Sinclair's assessment considered the risk associated with natural hazards in relation to the RMA:1991. For this site, the most relevant natural hazards are earthquake shaking, earthquake-induced land deformation (settlement and lateral stretch), the possible presence of undocumented fill (associated with historic soil disturbance) presenting a settlement risk, tsunami, and flood inundation.

■ Methodology

- a) Our desktop assessment comprises of a review of available online geotechnical data and records that are relevant to the site and the wider area.
- b) Eliot Sinclair has previously carried out significant investigation and reporting across the Pegasus area, hence, Eliot Sinclair is familiar with the local geotechnical conditions.

■ Key Findings

- a) The underlying ground model across the site is inferred to comprise of silt/sands with intermediate gravel layers within the upper 10m. It is likely the intermediate gravels taper out towards the south. Groundwater is expected to range between 1-2m below ground level.
- b) Analysis of the inferred underlying ground model across the site indicates TC2 land performance where there is a dense and thick gravel body within the upper 10m soil profile. The presence of any dense sand or gravel at least 3-4m thick is likely to suppress any deeper liquefiable soils and is not likely to contribute much to liquefaction-induced damage to shallow foundations. Therefore, TC2 land performance is expected across the northeast part of the site and TC3 across the southwest part of the site. We note, to fully assess the representative land performance across the site and to better map the transition between the two Technical Categories, additional geotechnical investigation and assessment is required.
- c) We have concluded that the risk and consequence of potential natural hazards is either acceptable or tolerable.

■ Recommendations

- a) Providing normal good practice investigation, design and development controls are implemented we have found no significant risks from natural hazards that would prevent the rezoning of the site.
- b) Any new foundations shall be subject to detailed investigation and design. We assess that it is likely that the proposed building structures can be supported on either shallow or deep foundations.

Contents

1. Introduction	1
2. Scope of Work	1
3. Site Description	1
4. Desktop Review	2
4.1. Engineering Geology	2
4.2. Land Classification	2
4.3. Historical Aerial Review	3
4.4. Tsunami Hazard Mapping	3
4.5. Flood Hazard Mapping	3
4.6. ECan Boreholes	4
4.7. NZGD – Nearby Site Investigation Records	5
4.8. Groundwater	5
5. Liquefaction Hazard	6
5.2. Vertical settlement due to liquefaction (index value)	7
5.6. Standard of Investigation	7
6. Site Specific Assessment of Risk from Natural Hazards	9
6.1. Conclusions	10
7. Preliminary Geotechnical Foundation Recommendations	10
7.1. Shallow foundations	10
7.2. Deep foundations	11
8. Disclaimer	11
Appendix A. Proposed Site Plan	
Appendix B. Site Photographs	
Appendix C. ECan Well Borehole Records	
Appendix D. NZGD Area Wide Deep Investigation Records and Site Plans	
Appendix E. CLiq Analysis	
Appendix F. GNS Risk Assessment Method	

1. Introduction

Eliot Sinclair & Partners Ltd was engaged by Dexin Investments Ltd to undertake a calibrated desktop assessment to support rezoning of the land at Pegasus Māketē - 1250 Main North Road, Woodend ('the site') and to provide preliminary geotechnical foundation recommendations.

In our preparation of the present report, we have relied exclusively on existing geotechnical investigations and our knowledge of the area.

Our calibrated desktop assessment will comment on the risk of natural hazards relevant to the site, as they relate to the proposed rezoning of the site.

2. Scope of Work

The scope of work for this assessment comprised:

- Review available data from the New Zealand Geotechnical Database¹ (NZGD), Canterbury Maps², Waimakariri District Council Natural Hazards Viewer³ and the Institute of Geological & Nuclear Sciences' (GNS) Active Faults Database⁴.
- Review over existing reporting and investigations relevant to the site.
- Undertake a calibrated desktop assessment (Level B) in accordance with MfE's '*planning and engineering guidance for potentially liquefaction-prone land*'.

Prepare a Desktop Natural Hazards Assessment Report to comment on the hazards relevant to the site, to summarise the general geotechnical conditions inferred across the site and to advise on preliminary geotechnical foundation recommendations.

3. Site Description

3.1. General

The site comprises a total area of approximately 3.05 hectares located on the corner of Main North Road and Pegasus Boulevard. The site is currently '*Rural*' in the operative Waimakariri District Council Plan with the proposed rezoning of the site as '*Special Purpose Zone*' to accommodate the '*Pegasus Resort*'.

The '*Special Purpose Zone*' will provide for a visitor destination to complement the Pegasus Resort activities, a range of agricultural tourism activities and some medium density residential development.

Refer to Appendix A for the proposed site plan.

The existing rural land comprise of a 170m² single storey dwelling constructed in the 1900s with an adjoining detached single garage and farm/garden sheds.

¹ New Zealand Geotechnical Database (NZGD) - <https://www.nzgd.org.nz/>

² Canterbury Maps - <https://mapviewer.canterburymaps.govt.nz>

³ <https://waimakariri.maps.arcgis.com/apps/MapSeries/index.html?appid=16d97d92a45f4b3081ffa3930b534553#>

⁴ GNS Active Faults Database - <http://maps.gns.cri.nz/website/af/viewer.htm>

The landform is typically flat with the 'Taranaki Stream (tributary of Rakahuri/Ashley River)' intersecting the site with a southwest to northeast orientation. We understand the stream is spring-fed. Based on existing LiDAR records, the invert of the stream typically ranges from around 1m to 2m below surrounding ground level.

Refer to Figure 1 for the site layout plan.

Refer to Appendix B for site photographs sourced from Harcourts, retrieved in December 2023.



Figure 1. Aerial imagery sourced from Eliot Sinclair GIS. Approximate site boundary outlined in yellow.

4. Desktop Review

4.1. Engineering Geology

The published geology⁵ for the site indicates it is underlain by 'Modern River floodplain/low-level degradation terrace. Unweathered, variably sorted gravel/sand/silt/clay. Surfaces <2-degree slope.' OIS1 (Holocene) river deposits.

The GNS Active Fault Database⁴ indicates there are no known active faults near the site.

4.2. Land Classification

The site is currently mapped as MBIE residential technical category 'N/A – Rural & Unmapped'. Technical category 'not applicable' means that the site has not been given a Technical Category.

⁵<https://data.gns.cri.nz/geology/>

The Technical Category (TC) system is intended for residential land only; however, it does provide a useful index of the relative vulnerability to liquefaction and earthquake induced land deformation for non-residential land.

The site is located within the 'Eastern Canterbury Liquefaction Susceptibility' zone as 'Liquefaction damage' is possible.

4.3. Historical Aerial Review

We have reviewed available historical aerial imagery sourced from Canterbury Maps and Google Earth dating back from the 1940s through to 2023. Our review has identified areas of soil disturbance across the north-eastern part of the site as a result of removing former building structures (detached sheds and outbuildings), trees and vegetation and the establishment of a market garden.

The existing stream has been clearly defined, dating back to the 1940s.

We note, there is a possibility uncontrolled fill may be encountered below existing ground level across the site.

4.4. Tsunami Hazard Mapping

Tsunami hazard mapping is provided by ECan⁶. The site is located outside (inland) of the three mapped Tsunami Evacuation Zones.

4.5. Flood Hazard Mapping

Eliot Sinclair has undertaken an Infrastructure Servicing Report⁷ which comments on the risk of flooding across the site following a 200-year Average Recurrence Interval (ARI) and a 50-year ARI storm event.

The report states the following:

- 200-year ARI
 - The site is subject to a low to medium flood hazard along the north of the site. The proposed buildings are located outside of the flood hazard area. Taranaki Stream is shown as a high hazard flood zone; however, this is due to stream depth. Refer to Figure 2.
- 50-year ARI
 - The Waimakariri District Natural Hazards Interactive Viewer does not provide a 50-year ARI flood model. However, it is reasonable to assume that the less extreme storm event will result in shallower depths.
 - The 50-year flood hazard maps will need to be requested from WDC or Environment Canterbury (ECan). It is recommended that building platforms be kept out of any flood zones where possible.
- Finished Floor Level Requirement
 - It is recommended that consultation directly with WDC is required to obtain the minimum floor level requirements for the site.

In conclusion, the proposed buildings shown in Appendix A are typically located outside of the 200 year ARI flood hazard model. Therefore, assuming normal good practice design and ensuring development controls are implemented, the risk of flooding following a 200-year ARI storm across the site is deemed acceptable for development.

⁶ <https://www.ecan.govt.nz/your-region/your-environment/natural-hazards/tsunamis/tsunami-evacuation-zones-and-warnings/>

⁷ Infrastructure Servicing Report at Pegasus Māketē, prepared for Dexlin Investments Limited, reference 503498.

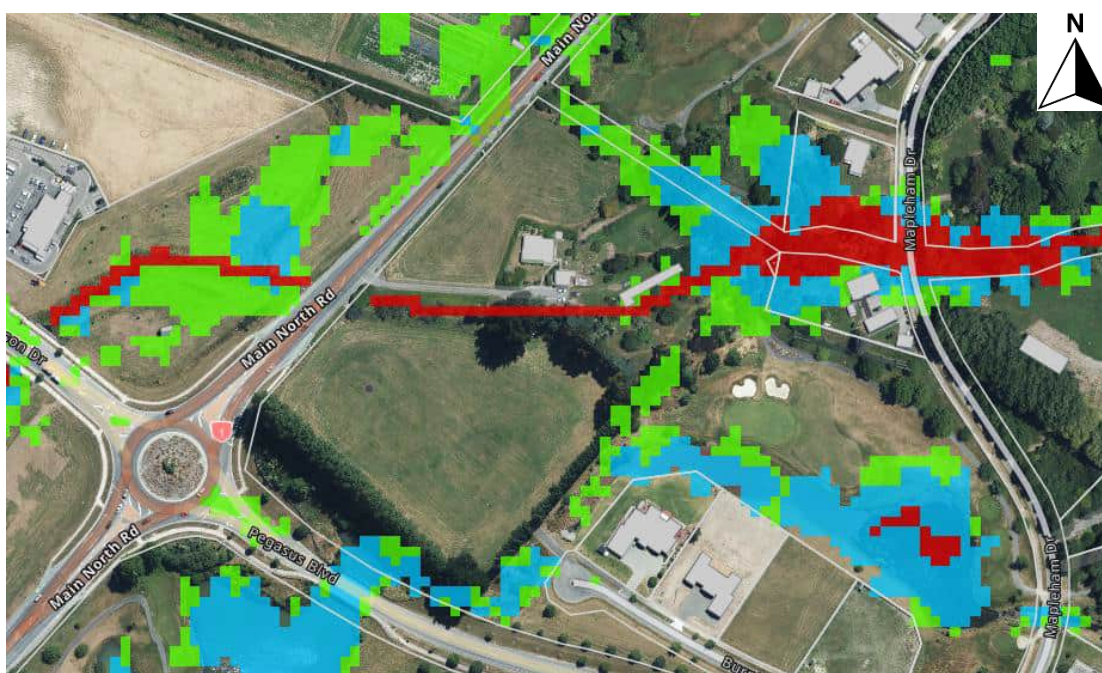


Figure 2. 200-year Flood Hazard Map sourced from Waimakariri District Natural Hazards Interactive Viewer retrieved in December 2023.

4.6. ECan Boreholes

Nearby ECan boreholes with well logs were reviewed from Canterbury Maps in December 2023. The well logs are summarised in Table 1 below and the bore locations are given in Figure 3. ECan borehole records are attached in Appendix C.

Table 1. Summary of ECan borehole well logs

Borehole ID	Depth of investigation (m)	Depth to upper gravel body (m)	Thickness of upper gravel layer (m)	Initial Water Level depth below Measuring Point (m)
M35/10715				
Water level observation bore	6.7	(Sand/Sandy SILT to Target depth)	(Sand/Sandy SILT to Target depth)	2.8
M35/10716				
Water level observation bore	9.7	(Sand/Sandy SILT to Target depth)	(Sand/Sandy SILT to Target depth)	4.6
M35/10720				
Water level observation bore	8.0	6.5	1.5	2.7



Figure 3. Aerial site plan with ECan well bores with borehole logs shown. Site is outlined in yellow.

4.7. NZGD – Nearby Site Investigation Records

At time of this report, there are existing geotechnical investigation records (i.e. Boreholes, Cone Penetrometer Test and test pits) shown on the NZGD (retrieved in December 2023).

Based on the wider area deep investigations, we infer the underlying ground model across the northeast part of the site is likely to be underlain by silty sand/sandy silt within the upper 3m to 6m soil profile underlain by dense sandy gravels and sands to at least 10m bgl.

However, at the southern part of the site, the underlying silty sand/sandy silt layers extends to at least 10m bgl.

Refer to Appendix D for the deep investigation ID and location plan.

4.8. Groundwater

Eliot Sinclair's review of existing geotechnical investigations across the surrounding area indicates groundwater is estimated to range around 1m to 2m bgl.

We note, the is site located within the 'Coastal Confined Gravel Aquifer System' sourced from Canterbury Maps.

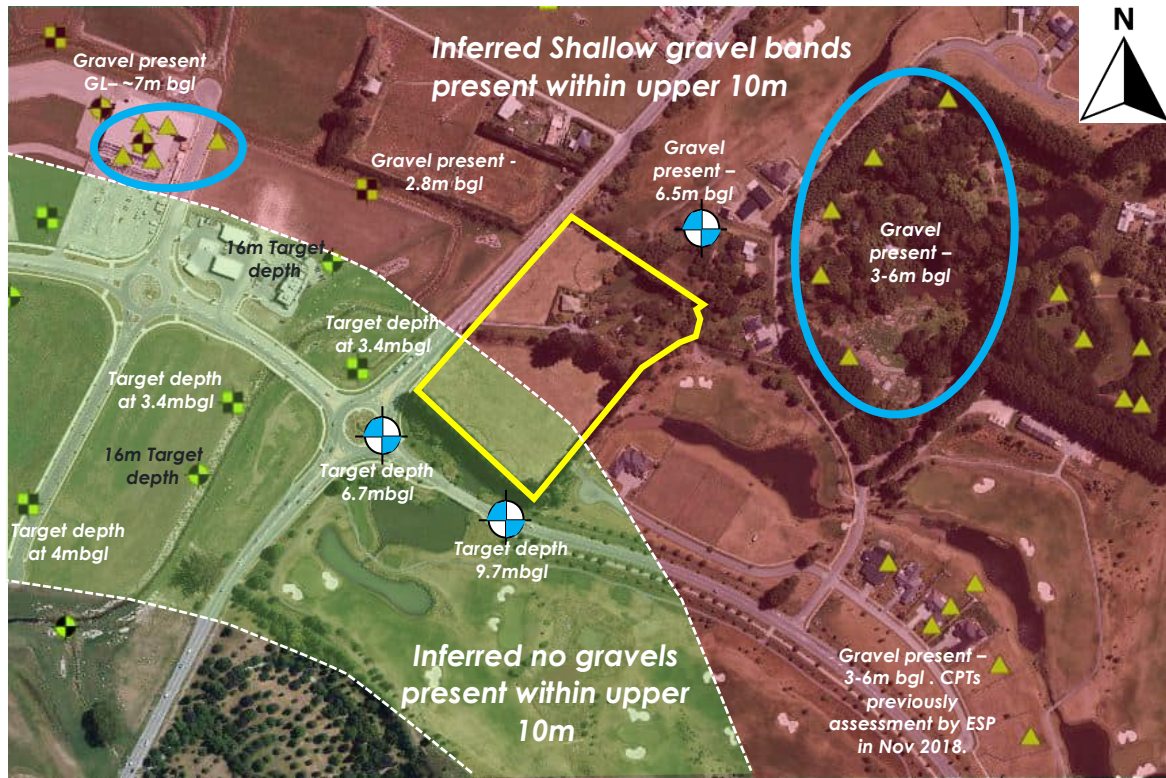


Figure 4. Deep test location plan sourced from the NZGD and ECan well bores with borehole logs. Test locations are approximate. CPTs outlined in blue were analysed.

5. Liquefaction Hazard

5.1. Assessment Method

The calculation of liquefaction triggering was undertaken using the method outlined in Boulanger & Idriss (2014)⁸, and the estimation of post-liquefaction induced settlements using the method outlined by Zhang et al (2002)⁹. The liquefaction analysis was calculated using CLiq¹⁰ software.

The CPTs surrounding the site were analysed for the Serviceability Limit State (SLS), Intermediate Limit State (ILS) and the Ultimate Limit State (ULS) levels of earthquake shaking in accordance with the New Zealand Geotechnical Society (NZGS) Module 1.

Based on the area wide investigations, we have assumed the 'earthquake' groundwater depth to be 2m bgl.

The CPTs analysed (refer to Figure 4) were sourced from 67 Mapleham Drive, approximately 100m northeast of the site and 10 Bob Robertson Drive, approximately 300m west of the site. Refer to Figure 4.

Refer to Appendix E our Liquefaction Analysis Report detailed results.

⁸ Boulanger, R. W., and Idriss, I. M. (2014). *CPT and SPT based liquefaction triggering procedures*. Report No. UCD/CGM-14/01, Center for Geotechnical Modeling, Department of Civil and Environmental Engineering, University of California, Davis, CA, 134 pp.

⁹ Zhang, G., Robertson, P.K. & Brachman, R. (2002). *Estimating liquefaction-induced ground settlements from CPT for level ground*. Canadian Geotechnical Journal, 39(5): 1168-1180.

¹⁰ CLiq (version 2.3.1.14). GeoLogismiki Geotechnical Software

5.2. Vertical settlement due to liquefaction (index value)

The liquefaction-induced 'index' settlement values (S_{VID}) were calculated using the method by Zhang et al (2002)¹² for a range of parameters that are estimated from the four basic CPT parameters (depth, cone tip resistance, skin friction and pore water pressure) and represent 'free-field' settlements. Therefore, the settlements shown in Table 2 are not an exact figure, but only index values for interpretation of relative susceptibility to damaging effect of liquefaction.

Summary plots of the vertical settlement results are included in Appendix E.

5.3. Liquefaction Severity Number (LSN)

The liquefaction severity number (LSN) is a parameter developed to predict the more damaging effects of shallow liquefaction on residential land and shallow foundations. Calculation of the LSN is limited to the upper 10m of soil.

The CPT's where gravels were encountered at depth, calculated LSN values for SLS and ULS range between 0-20 indicating little to minor expression of liquefaction.

5.4. Lateral Stretch

Lateral stretch is possible close to the existing stream within the site. This risk can be mitigated through site specific investigation, design, and development controls.

5.5. Technical Category

The Technical Category (TC) system is intended for residential land only; however, it does provide a useful index of the relative vulnerability to liquefaction and earthquake induced land deformation for non-residential land.

Our desktop assessment of the earthquake-induced land performance finds the northeast part of the site is expected to have TC2 equivalent earthquake performance defined as '*minor to moderate land damage from liquefaction is possible in future large earthquakes*'. This area is expected to have a shallow dense gravel body within the upper 10m soil profile. The presence of any dense sand or gravel at least 3-4m thick is likely to suppress any deeper liquefiable soils and is not likely to contribute much to liquefaction-induced damage to shallow foundations.

At the southeast part of the site, the underlying inferred silt/sands are likely to extend at least 10m bgl and is inferred to have a TC3 equivalent land performance defined as '*moderate to significant land damage from liquefaction is possible in future large earthquakes*'.

Refer to Figure 5 for the area extent of the inferred land performance areas.

We note, to fully assess the representative land performance across the site and to better map the transition between the two Technical Categories, additional geotechnical investigation and assessment is required.

5.6. Standard of Investigation

In terms of Table 3.1 of MfE's guidance, Eliot Sinclair's desktop assessment of the site and surrounding areas can be considered to satisfy the requirements of Level B *i.e Calibrated desktop assessment*.

Location	CPT ID	Depth of CPT test (m bgl)	Predrill (m bgl)	Liquefaction-induced 'index' settlements, S_{VID} (mm)			MBIE Equivalent land classification at test location
				SLS (M7.5, 0.13g)	SLSa (M6.0, 0.19g)	ULS (M7.5, 0.35g)	
67 Mapleham Drive	CPT_37505	12.2	-	5	13	70	TC2
67 Mapleham Drive	CPT_37930*	5.2	-	1	4	29	TC2*
67 Mapleham Drive	CPT_37929*	2.8	-	-	-	-	-
67 Mapleham Drive	CPT_37927*	9.2	-	10	18	43	TC2*
67 Mapleham Drive	CPT_37926*	6.2	-	7	15	31	TC2*
10 Bob Robertson Drive	CPT_201450*	7.0	GL - 2.4	0	1	11	TC1*
10 Bob Robertson Drive	CPT_126066*	7.7	0.8 – 3.6	6	12	15	TC1*
10 Bob Robertson Drive	CPT_126063	15	0.7 – 3.2	19	34	73	TC2
10 Bob Robertson Drive	CPT_126065*	6.3	GL – 3.7	1	4	11	TC1*
10 Bob Robertson Drive	CPT_126062	15	0.7 – 3.1	50	78	97	TC3
10 Bob Robertson Drive	CPT_126060*	7.7	GL – 3.1	3	8	22	TC1

Table 2. Overall Vertical Settlements Summary Table – Limited to the upper 10m soil profile

Notes:

*Shallow refusal of CPT indicating dense ground at the terminated depth, but analysis is for less than 10m penetration. Where penetration was $\leq 3m$, data was not analysed.

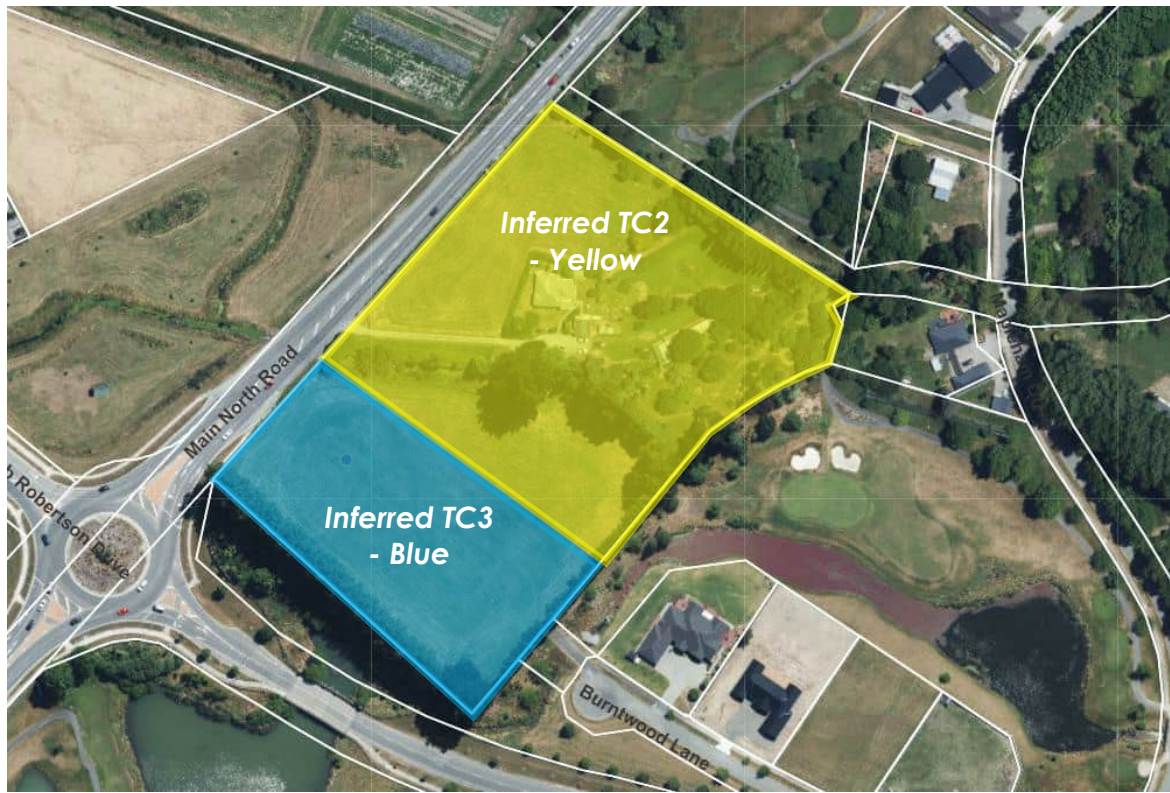


Figure 5. Inferred land performance in accordance with Eliot Sinclair's wider area desktop review and MBIE guidance.

6. Site Specific Assessment of Risk from Natural Hazards

Council can refuse plan change consent if there is a significant risk of material damage or injury from one, or a combination of, natural hazards. Decision-makers are guided by the matters set out in RMA Section 106 which requires an assessment of:

- The existing likelihood and consequence of natural hazards, and
- The likelihood that subdivision of the site could increase the likelihood or consequence of the natural hazard.

Refer to Appendix F for GNS risk assessment method.

The RMA defines natural hazards as:

Any atmospheric or earth or water related occurrence (including earthquake, tsunami, erosion, volcanic and geothermal activity, landslip, subsidence, sedimentation, wind, drought, fire, or flooding) the action of which adversely affects or may adversely affect human life, property, or other aspects of the environment¹¹.

¹¹ <https://www.mfe.govt.nz/publications/rma/resource-legislation-amendments-2017-fact-sheet-series>

Our desktop risk assessment of natural hazards for the proposed development assuming normal good practice investigation, design and development controls are implemented is listed below;

- Earthquake Fault Rupture and shaking hazard (Tolerable).
 - The nearest mapped active fault is located kilometres away to the northwest.
- Tsunami (Tolerable)
 - The site is outside of the three mapped Tsunami Evacuation Zones.
- Erosion & Sedimentation (Acceptable)
 - No erosion or sedimentation was observed following review of available site photographs and aerial imagery.
- Subsidence (Tolerable)
 - The risk of subsidence from liquefaction across the site is considered to be moderate and is inferred to be equivalent to TC2 for future ground performance within the northeast part of the site and TC3 across the southwest part of the site. To fully assess the representative land performance across the site, additional geotechnical investigation and assessment is required.
 - The risk of subsidence from undocumented fill following normal good practice investigation, design, and construction management practices the site can be engineered for suitability.
- Flooding (Acceptable)
 - The proposed buildings are located outside of the 200-year ARI flood hazard model hence the risk of flooding following a 200-year ARI storm is acceptable.

6.1. Conclusions

We have considered the risk associated with natural hazards in relation to the RMA:1991 and concluded that the risk and consequence of potential natural hazards is either acceptable or tolerable. For this site, the most relevant natural hazards are earthquake shaking, earthquake-induced land deformation (settlement and lateral stretch), the possible presence of undocumented fill (associated with historic soil disturbance) presenting a settlement risk, and flood inundation.

Providing normal good practice investigation, design and development controls are implemented we have found no significant risks from natural hazards that would prevent the rezoning of the site.

7. Preliminary Geotechnical Foundation Recommendations

Eliot Sinclair's recommendations below are strictly preliminary, and any new foundations shall be subject to detailed investigation and design.

7.1. Shallow foundations

The risk of differential settlement to a tolerable level could be mitigated via shallow ground improvement in conjunction with a stiffened raft, grid ground beams or mat-type foundations.

Whilst there is some risk of differential settlement occurring in a SLS and an ULS event, the resilient shallow foundations above are easily able to be re-levelled using a ground injection method if there is ground deformation and foundation movement.

7.2. Deep foundations

Eliot Sinclair's assessment of the underlying deeper soils is prone to liquefaction-induced settlements and lateral displacement. Dense gravel layers are inferred to be underlain across the northeast part of the site.

If there is a requirement to reduce the risk of differential settlement further, any new foundations shall be supported on deep ground improvement (i.e. stone columns) or deep piles (i.e. screw or driven).

Site specific deep investigations will be required to confirm depth and feasibility of any proposed ground improvement methods.

8. Disclaimer

This report has been prepared by Eliot Sinclair & Partners Limited ("Eliot Sinclair") only for the intended purpose as a Desktop Natural Hazards Risk Assessment.

The report is based on:

- Information shown on the NZGD, Canterbury Maps, Waimakariri District Council Natural Hazards Viewer and GNS's Active Faults Database.
- Review over existing reporting and investigations relevant to the site.
- Eliot Sinclair's calibrated desktop assessment (Level B) in accordance with MfE's '*planning and engineering guidance for potentially liquefaction-prone land*'.

Where data supplied by Dexin Investments Ltd or other external sources, including previous site investigation reports, have been relied upon, it has been assumed that the information is correct unless otherwise stated. No responsibility is accepted by Eliot Sinclair for incomplete or inaccurate data supplied by other parties.

Whilst every care has been taken during our desktop assessment and interpretation of the subsurface conditions to ensure that the conclusions drawn, and the opinions and recommendations expressed are correct at the time of reporting, Eliot Sinclair has not performed an assessment of all possible conditions or circumstances that may exist at the site. Variations in conditions may occur between investigatory locations and there may be conditions such as subsoil strata and features that were not detected by the scope of the investigation that was carried out or have been covered over or obscured over time. Additionally, on-going seismicity in the general area may lead to deterioration or additional ground settlement that could not have been anticipated at the time of writing this report. Eliot Sinclair does not provide any warranty, either express or implied, that all conditions will conform exactly to the assessments contained in this report.

The exposure of conditions that vary from those described in this report, or occurrence of additional strong seismicity, or any future update of MBIE's guidelines may require a review of our assessment. Eliot Sinclair should be contacted to confirm the validity of this report should any of these occur.

This report has been prepared for the benefit of Dexin Investments Ltd for the purposes as stated above. This report is specifically prepared for the proposed rezoning of the site. No liability is accepted by Eliot Sinclair or any of their employees with respect to the use of this report, in whole or in part, for any other purpose or by any other party.

Appendix A. Proposed Site Plan

Proposed Site Plan

- With Flood Risk



Appendix B. Site Photographs



Appendix C. ECan Well Borehole Records



Bore or Well No	M35/10720
Well Name	BTWN GLADSTONE & PREECES ROADS
Owner	PEGASUS TOWN LIMITED

Well Number	M35/10720	File Number	C06C/22855
Owner	PEGASUS TOWN LIMITED	Well Status	Active (exist, present)
Street/Road	BTWN GLADSTONE & PREECES ROADS	NZTM Grid Reference	BW24:73889-04939
Locality	Woodend	NZTM X and Y	1573889 - 5204939
Location Description		Location Accuracy	2 - 15m
CWMS Zone	Waimakariri	Use	Water Level Observation,
Groundwater Allocation Zone	Ashley	Water Level Monitoring	--
Depth	8.00m	Water Level Count	0
Diameter	32mm	Initial Water Level	2.70m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	8.12m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 0.5 m	First reading	
Ground Level	0.10m below MP	Last reading	
Strata Layers	4	Calc Min 80%	2.49m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	0
Drill Date	18 May 2005	Max Tested Yield	
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	
Drilling Method	Driven Pipe	Specific Capacity	
Casing Material	PVC	Last Updated	29 Jun 2023
Pump Type		Last Field Check	
Water Use Data	No		

Screens

SCREEN NO.	SCREEN TYPE	TOP (M)	BOTTOM (M)	SLOT SIZE (MM)	SLOT LENGTH (MM)	DIAMETER (MM)	LEADER LENGTH (MM)
1	Slotted PVC	5	8				

No step tests for this well

Comments

COMMENT DATE	COMMENT
01 May 2006	Gridref changed from: M35:8402-6678
01 May 2006	Also called PT-019 on BCR
10 Jan 2023	Reference Level updated using LIDAR imagery in Dec 2022. The existing RL was 9.23 and the QAR RL was 4. The method of calculating the original RL was: Interpolated DTM. If GL from MP is updated in future please assess if RL also needs to be updated.

Bore Log

Borelog for well M35/10720

Grid Reference (NZTM): 1573886 mE, 5204640 mN
 Location Accuracy: 2 - 15m
 Ground Level Altitude: 8.0 m +MSD Accuracy: < 0.5 m
 Driller: McMillan Drilling Ltd
 Drill Method: Driven Pipe
 Borelog Depth: 8.0 m Drill Date: 18 May 2005



Stratigraphic Level	Water Level	Depth (m)	Full Driller's Description	Formation Code
		0.20m	dark brown silty yellow brown silty fine sandy fine sandy silt - mottled non plastic	
		3.00m	yellow brown fine-medium sand with some silty fine-medium sand with rare wood fragments - saturated, non to slightly plastic	
		6.50m	bluish grey sandy gravel with some silt - well graded, saturated, non angular to sub rounded fine to coarse phyllonite	
		8.00m		



Bore or Well No	M35/10715
Well Name	BTWN GLADSTONE & PREECES ROADS
Owner	PEGASUS TOWN LIMITED

Well Number	M35/10715	File Number	C06C/22855
Owner	PEGASUS TOWN LIMITED	Well Status	Active (exist, present)
Street/Road	BTWN GLADSTONE & PREECES ROADS	NZTM Grid Reference	BW24:73591-04745
Locality	Woodend	NZTM X and Y	1573591 - 5204745
Location Description		Location Accuracy	2 - 15m
CWMS Zone	Waimakariri	Use	Water Level Observation,
Groundwater Allocation Zone	Ashley	Water Level Monitoring	--
Depth	6.70m	Water Level Count	0
Diameter	32mm	Initial Water Level	2.80m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	9.51m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 0.5 m	First reading	
Ground Level	0.10m below MP	Last reading	
Strata Layers	11	Calc Min 80%	2.37m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	0
Drill Date	12 Apr 2005	Max Tested Yield	
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	
Drilling Method	Driven Pipe	Specific Capacity	
Casing Material	PVC	Last Updated	29 Jun 2023
Pump Type		Last Field Check	
Water Use Data	No		

Screens

SCREEN NO.	SCREEN TYPE	TOP (M)	BOTTOM (M)	SLOT SIZE (MM)	SLOT LENGTH (MM)	DIAMETER (MM)	LEADER LENGTH (MM)
1	Slotted PVC	3.3	6.3				

No step tests for this well

Comments

COMMENT DATE	COMMENT
01 May 2006	Gridref changed from: M35:8361-6637
01 May 2006	Also called PT-014 on BCR
10 Jan 2023	Reference Level updated using LIDAR imagery in Dec 2022. The existing RL was 10.57 and the QAR RL was 4. The method of calculating the original RL was: Interpolated DTM. If GL from MP is updated in future please assess if RL also needs to be updated.

Bore Log

Borelog for well M35/10715

Grid Reference (NZTM): 1573591 mE, 5204746 mN
 Location Accuracy: 2 - 15m
 Ground Level Altitude: 0.4 m +MSD Accuracy: < 0.5 m
 Driller: McMillan Drilling Ltd
 Drill Method: Driven Pipe
 Borelog Depth: 6.7 m Drill Date: 12-Apr-2005



Stratigraphic Unit	Water Level	Depth (m)	Full Driller's Description	Formation Code
		0.20m	topsoil	
		0.20m - 0.80m	light grey brown silty fine-medium sand - dry	
		0.80m - 1.20m	grey mottled orange silt with some clay - stiff, moist non-plastic	
		1.20m - 1.80m	brown fine-medium sand - moist	
		1.80m - 2.20m	grey mottled orange silt with some clay - soft, saturated, moderately plastic	
		2.20m - 2.85m	dark orange brown medium to coarse sand - saturated	
		2.85m - 3.00m	light grey clayey silt - very soft, saturated highly plastic	
		3.00m - 4.25m	grey with orange brown mottled silt fine sand - very soft, saturated moderately plastic	
		4.25m - 4.35m	grey with orange brown mottled fine sand with more silt - loosely packed, saturated non plastic	
		4.35m - 5.70m	blue grey fine sand with some silt - tightly packed, saturated non plastic	
		5.70m - 6.70m	grey sand, choked at 6.7m due to sand filling in the bore	



Bore or Well No	M35/10716
Well Name	BTWN GLADSTONE & PREECES ROADS
Owner	PEGASUS TOWN LIMITED

Well Number	M35/10716	File Number	C06C/22855
Owner	PEGASUS TOWN LIMITED	Well Status	Active (exist, present)
Street/Road	BTWN GLADSTONE & PREECES ROADS	NZTM Grid Reference	BW24:73712-04671
Locality	Woodend	NZTM X and Y	1573712 - 5204671
Location Description		Location Accuracy	2 - 15m
CWMS Zone	Waimakariri	Use	Water Level Observation,
Groundwater Allocation Zone	Ashley	Water Level Monitoring	--
Depth	9.70m	Water Level Count	0
Diameter	32mm	Initial Water Level	4.55m below MP
Measuring Point Description	ToC	Highest Water Level	
Measuring Point Elevation	9.57m above MSL (Lyttelton 1937)	Lowest Water Level	
Elevation Accuracy	< 0.5 m	First reading	
Ground Level	0.10m below MP	Last reading	
Strata Layers	10	Calc Min 80%	2.39m below MP (Estimated)
Aquifer Name		Aquifer Tests	0
Aquifer Type		Yield Drawdown Tests	0
Drill Date	13 Apr 2005	Max Tested Yield	
Driller	McMillan Drilling Ltd	Drawdown at Max Tested Yield	
Drilling Method	Driven Pipe	Specific Capacity	
Casing Material	PVC	Last Updated	29 Jun 2023
Pump Type		Last Field Check	
Water Use Data	No		

Screens

SCREEN NO.	SCREEN TYPE	TOP (M)	BOTTOM (M)	SLOT SIZE (MM)	SLOT LENGTH (MM)	DIAMETER (MM)	LEADER LENGTH (MM)
1	Slotted PVC	2.8	5.8				

No step tests for this well

Comments

COMMENT DATE	COMMENT
01 May 2006	Gridref changed from: M35:8371-6630
01 May 2006	Also called PT-015 on BCR
10 Jan 2023	Reference Level updated using LIDAR imagery in Dec 2022. The existing RL was 10.38 and the QAR RL was 4. The method of calculating the original RL was: Interpolated DTM. If GL from MP is updated in future please assess if RL also needs to be updated.

Bore Log

Borelog for well M35/10716

Grid Reference (NZTM): 1573712 mE, 5204672 mN
 Location Accuracy: 2 - 15m
 Ground Level Altitude: 9.5 m -MSD Accuracy: < 0.5 m
 Driller: McMillan Drilling Ltd
 Drill Method: Driven Pipe
 Borelog Depth: 9.7 m Drill Date: 13 Apr 2005



Strat (m)	Water Level	Depth (m)	Full Driller's Description	Formation Code
		0.30m	topsoil light grey brown silty fine to medium sand - bossy packed, dry	
		1.85m	grey mottled orange brown fine sandy silt - lightly packed, moist, non-plastic	
		2.25m	brassy grey fine to medium sand - rapid, coarsening to medium coarse sand at 3.0m	
		3.25m	grey mottled orange fine sandy silt - soft saturated slightly plastic	
		4.30m	coarsening to fine to medium sand with some silt	
		4.80m	blue grey silty fine sand with some silt - saturated, slightly plastic	
		5.70m	blue grey silty fine sand - saturated, slightly plastic	
		6.70m	fine to fine sandy silt - very soft, saturated, moderately plastic, slightly organic	
		7.70m	fine to fine sandy silt - very soft, saturated, moderately plastic, slightly organic, below 8.7m becomes soft/fine	
		9.70m		

Appendix D. NZGD Area Wide Deep Investigation Records and Site Plans



TPL147435

TPL147434

TPL147429



TONKIN & TAYLOR LTD

EXCAVATION LOG

EXCAVATION No: TP9-04
Hole Location: See Attached Plan
SHEET 1 OF 1

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831272.75 mN 395708.94 mE	EXPOSURE TYPE: Test Pit	EXCAV. STARTED: 12/2/14
R.L. 9.30 m	EQUIPMENT: Tracked Excavator 8 tonne	EXCAV FINISHED: 12/2/14
DATUM Mount Pleasant (2000)	OPERATOR: Shearings	LOGGED BY: JXXM
	DIMENSIONS: 1.75m x 3.7m x 3.4m	CHECKED BY: RRG

EXCAVATION TESTS				ENGINEERING DESCRIPTION				GEOLOGICAL						
PENETRATION 1 2 3	SUPPORT	WATER	SAMPLES, TESTS	R.L. (m) DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	SOIL NAME, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, SECONDARY AND MINOR COMPONENTS	MOISTURE CONDITION /WEATHERING	STRENGTH/DENSITY CLASSIFICATION	ESTIMATED SHEAR STRENGTH (kPa)			ORIGIN TYPE, MINERAL COMPOSITION, DEFECTS, STRUCTURE	UNIT
										10	20	100		
				9.0	[Symbol]	ML	TOPSOIL: Sandy SILT, with minor rootlets; brownish grey. Firm, dry; sand, fine; rootlets, fine.	D	F				Topsoil	
				8.5	[Symbol]	SM	Silty, very fine SAND, with trace rootlets; yellowish grey. Dry; rootlets, fine. 0.45m: becomes trace iron staining, moist.	M					Springston Formation	
			Bag 1	8.0	[Symbol]	ML	SILT, with some sand and trace rootlets; brownish grey, with trace iron staining. Firm, moist, non-plastic; sand, fine; rootlets, fine. 0.8m: grades to minor sand, low plasticity. 0.9m: grades to sandy, non-plastic.		F					
				7.5	[Symbol]	SP	Fine to medium SAND, with some silt; brownish grey, trace iron staining. Moist. 1.8m: becomes wet. 1.9m: Slight Water Seepage.	W						
				7.0	[Symbol]									
				6.5	[Symbol]	ML	SILT, with minor clay; bluish grey. Very soft, wet, moderate plasticity, non-dilatant. 2.75m: becomes saturated.		VS					
			Bag 2	6.0	[Symbol]			Sat						
				5.5	[Symbol]		END OF TEST PIT AT 3.4m.							
				5.0	[Symbol]		Could not advance further due to significant collapse. Collapse worsened due to water inflow below 2.4m.							

T-T DATATEMPLATE.GDT_rfg



TONKIN & TAYLOR LTD

EXCAVATION LOG

EXCAVATION No: TP9-05
Hole Location: See Attached Plan
SHEET 1 OF 1

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831429.77 mN 395717.78 mE	EXPOSURE TYPE: Test Pit	EXCAV. STARTED: 13/2/14
R.L. 9.30 m	EQUIPMENT: Tracked Excavator 8 tonne	EXCAV FINISHED: 13/2/14
DATUM Mount Pleasant (2000)	OPERATOR: Shearings	LOGGED BY: JXXM
	DIMENSIONS: 1.75m x 3.9m x 3.8m	CHECKED BY: RRG

EXCAVATION TESTS	ENGINEERING DESCRIPTION	GEOLOGICAL
------------------	-------------------------	------------

PENETRATION	SUPPORT	WATER	SAMPLES, TESTS	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	SOIL NAME, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, SECONDARY AND MINOR COMPONENTS	MOISTURE CONDITION / WEATHERING		STRENGTH / DENSITY CLASSIFICATION			ESTIMATED SHEAR STRENGTH (kPa)	ORIGIN TYPE, MINERAL COMPOSITION, DEFECTS, STRUCTURE	UNIT
									D	M	W	St	F			
1 2 3						x	ML	TOPSOIL: Sandy SILT, with trace rootlets; brownish grey. Stiff, dry; rootlets, fine; sand, fine.	D			St	10 20 100 200	Topsoil		
					9.0	x	ML	Sandy SILT, with trace rootlets; brownish grey with trace iron staining. Stiff, dry, non-plastic; rootlets, fine; sand, fine.						Springston Formation		
					0.5	x	SM	Grades to silty, very fine SAND; brownish grey with trace iron staining. Dry.								
					0.65m: becomes moist.	x	ML	Grades to SILT, with some sand; brownish grey with minor iron staining. Firm, moist, non-plastic; sand, fine.	M			F				
					1.0	x	SM	Grades to silty, very fine SAND; brownish grey with minor iron staining. Moist, non-plastic. Minor sandy silt layers/lenses.								
				Bag 1												
					8.0											
				Bag 2												
					1.5											
					2.0											
					2.2m: becomes trace iron staining.											
					2.5											
					3.0		GW	Sandy, fine to coarse GRAVEL, with trace silt; reddish brown, dominant iron staining. Loose, moist; sand, fine to coarse; gravel, sub-angular to sub-rounded.				L				
					3.5											
					3.7m: becomes grey, iron staining absent. Wet.											
					3.8m											
					4.0			END OF TEST PIT AT 3.8m.								
					4.5			Could not advance further, as excavator at maximum reach.								
					5.0			No groundwater encountered.								
					5.5											

T-T DATA TEMPLATE: GDT.rtg



TONKIN & TAYLOR LTD

EXCAVATION LOG

EXCAVATION No: TP8-01

Hole Location: See Attached Plan

SHEET 1 OF 1

PROJECT: Ravenswood Subdivision LOCATION: Ravenswood, Woodend JOB No: 53214.100

CO-ORDINATES: 831242.95 mN 395599.78 mE EXPOSURE TYPE: Test Pit EQUIPMENT: Tracked Excavator 8 tonne EXCAV. STARTED: 12/2/14

R.L. 9.70 m OPERATOR: Shearings LOGGED BY: JXXM

DATUM Mount Pleasant (2000) DIMENSIONS: 1.75m x 3.7m x 3.88m CHECKED BY: RRG

EXCAVATION TESTS ENGINEERING DESCRIPTION GEOLOGICAL

PENETRATION 1 2 3	SUPPORT WATER	SAMPLES, TESTS	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	SOIL NAME, PLASTICITY OR PARTICLE SIZE CHARACTERISTICS, COLOUR, SECONDARY AND MINOR COMPONENTS	MOISTURE CONDITION / WEATHERING	STRENGTH / DENSITY CLASSIFICATION	ESTIMATED SHEAR STRENGTH (kPa)			ORIGIN TYPE, MINERAL COMPOSITION, DEFECTS, STRUCTURE	UNIT
										10	20	100		
				9.5			TOPSOIL: Sandy SILT, with trace rootlets; brownish grey. Stiff; sand, fine; rootlets, fine.	D	St				Topsoil	
				0.5		SM	Silty, very fine SAND, with trace rootlets; yellowish grey. Dry; rootlets, fine.						Springston Formation	
				9.0		SP	0.45m: grades to some silt. Becomes brownish grey, trace iron staining, loose, moist.	M	L					
						SM	0.6m: grades to silty.							
						ML	Grades to sandy SILT, with trace rootlets; brownish grey with trace iron staining. Firm, moist, non-plastic; sand, fine; rootlets, fine.		F					
						SM	0.8m: grades to minor sand. Stiff, low plasticity. Silty, fine SAND, with trace rootlets; brownish grey, trace iron staining. Moist; rootlets, fine.		St					
						SP	1.4m: becomes wet. Grades to some silt. Minor iron staining. Rootlets absent.							
				8.0										
				7.5										
				2.0		ML	SILT; grey with orange mottles and trace iron staining. Soft, wet, low to moderate plasticity, very slow. Slight Water Seepage.		S					
				3.0										
				6.5										
				3.5			3.3m: grades to minor clay. Becomes bluish grey, iron staining and orange mottles absent. Very soft, moderate plasticity, non-dilatant.		VS					
				6.0										
				4.0			END OF TEST PIT AT 3.8.							
				5.5			Could not advance further, as excavator at maximum reach.							
				4.5										
				5.0										

T-T DATA TEMPLATE: GDT.rtg

Log Scale 1:25

EXCAVATION 53214TP.GPJ 25-Mar-2014



BH_147392

BH_126061

BH_147393

BH_147390

BH_147388

BH_147389

38598

60m

200ft



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH9-02

Hole Location: See Attached Plan

SHEET 1 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831502.81 mN 395484.38 mE	DRILL TYPE: AMS-17C	HOLE STARTED: 27/1/14
R.L.: 9.40 m	DRILL METHOD: SONIC	HOLE FINISHED: 27/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JXXM CHECKED: RRG

GEOLOGICAL						ENGINEERING DESCRIPTION																			
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)					COMPRESSIVE STRENGTH (MPa)					DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
														0	25	50	100	200	0	50	100	200	500		
Topsoil																								TOPSOIL: SILT, with trace sand and rootlets; brownish dark grey. Soft, moist; sand, fine.	
Springston Formation			47	SONIC					9.0		ML	M	S											SILT, with some sand; brownish grey, with trace iron staining. Firm, dry; sand, fine.	
									0.5		SM	M	L											Silty, fine SAND; brownish grey, with trace iron staining. Loose, moist.	
									0.55m		SP													Grades to some silt.	
									0.65m															Grades to minor silt.	
									NO RECOVERY															0.8-1.5m.	
									1.5		SM													Grades to silty, trace wood fragments.	
			100	SPT		1/1/2/ 0/0/0 N=2			1.65m															Becomes grey, iron staining absent.	
									1.75m		ML	W	S											SILT, with trace organics, some sand; grey. Soft, wet, low plasticity, quick; sand, fine; organics, fibrous.	
									2.2m															Grades to sandy. Non-plastic.	
			100	SONIC					2.5		SM		L											Silty, fine SAND, trace organics; grey. Loose, wet; organics, fibrous. Interbedded SILT layers within sand.	
									3.0		GW		MD											Fine to coarse GRAVEL, with some sand and trace silt; grey. Medium dense, wet; sand, fine to coarse; gravel, sub-angular to sub-rounded.	
			67	SPT		5/6/7/ 6/7/7 N=27			3.3m															NO RECOVERY: 3.3-3.45m.	
									3.45m															Grades to trace cobbles.	
			91	SONIC					4.1m															Grades to minor sand.	
									4.5															NO RECOVERY: 4.5-4.6m.	
			22	SPT		3/2/2/ 1/2/2 N=7			4.6m				L											Becomes loose.	
									4.7-5.0m															NO RECOVERY: 4.7-5.0m.	

T-T DATATEMPLATE.GDT.rtg

Log Scale 1:25

BORELOG 53214-A.GPJ 25-Mar-2014



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH9-02
 Hole Location: See Attached Plan
 SHEET 2 OF 4

PROJECT: Ravenswood Subdivision		LOCATION: Ravenswood, Woodend		JOB No: 53214.100										
CO-ORDINATES: 831502.81 mN 395484.38 mE		DRILL TYPE: AMS-17C		HOLE STARTED: 27/1/14										
R.L.: 9.40 m		DRILL METHOD: SONIC		HOLE FINISHED: 27/1/14										
DATUM: Mount Pleasant (2000)		DRILL FLUID: Drill pro		DRILLED BY: Prodrill Ltd										
				LOGGED BY: JXXM CHECKED: RRG										
GEOLOGICAL			ENGINEERING DESCRIPTION											
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES R.L. (m) DEPTH (m) GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE CONDITION	WEATHERING	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)	COMPRESSIVE STRENGTH (MPa)	DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
Springston Formation		100	SONIC		1/1/2/ 1/1/0 N=4	4.0	GW	W	L					Fine to coarse GRAVEL, with minor sand and trace silt and cobbles; grey. Loose, wet; sand, fine to coarse; gravel, sub-angular to sub-rounded. NO RECOVERY: 5.0-5.05m. 5.25m: Grades to minor wood fragments. Saturated. 5.35m: Grades to wood fragmnets absent.
		67	SPT			5.5		Sat						
		100	SONIC			6.0		W						5.9m: Grades to some sand. Wet.
		67	SPT			6.5	SP							Fine SAND, with some silt and trace wood fragments; grey. Loose, wet. NO RECOVERY: 6.4-6.55m.
		100	SONIC			7.0	SM ML		VS					6.8m: Grades to silty. SILT, with some sand and trace organics; grey. Very soft, wet, low plasticity, quick; sand, fine; organics, fibrous.
		100	SPT		3/1/0/ 1/3/5 N=9	7.5	SM		L					Silty, fine SAND; grey. Loose, wet. WOOD; brown. Wet.
		100	SPT			8.0	ML		VS					SILT, with some sand; grey. Very soft, wet, low plasticity, quick; sand, fine. WOOD; brown. Wet.
		67	SPT		0/0/0/ 0/0/1 N=1	8.5	ML		F					SILT, with some sand; grey. Firm, wet, non-plastic, quick; sand, fine. 8.55m: Grades to sand absent. Soft, moderate plasticity, non-dilatant.
		91	SONIC			9.0			S					9.1m: Grades to trace fibrous organics.
						9.5								NO RECOVERY: 9.4-9.55m. 9.55m: Becomes low to moderate plasticity. Very slow.

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH9-02
 Hole Location: See Attached Plan
 SHEET 3 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831502.81 mN 395484.38 mE	DRILL TYPE: AMS-17C	HOLE STARTED: 27/1/14
R.L.: 9.40 m	DRILL METHOD: SONIC	HOLE FINISHED: 27/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JXXM CHECKED: RRG

GEOLOGICAL										ENGINEERING DESCRIPTION															
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)					COMPRESSIVE STRENGTH (MPa)					DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
														0	25	50	100	200	50	100	200	500	1000		
Springston Formation			91	SONIC		0/0/0/ 0/0/0 N=0				ML	W	S											SILT, with trace organics; grey. Soft, wet, low to moderate plasticity, very slow; organics, fibrous.		
			100	SPT				-1.0				VS											10.35m: Becomes very soft. Grades to minor organics. 10.5		
			100	SPT				-1.5															NO RECOVERY: 10.6-10.7m. 11.0		
			100	SONIC				-2.0			SM	L											Silty, fine SAND, with trace organics; grey. Loose, wet; organics, fibrous.		
			100	SONIC				-2.5			ML	S											Grades to sandy SILT, with trace organics; 11.5 grey. Soft, wet, non-plastic, quick; sand, fine; organics, fibrous.		
			100	SPT		3/6/9/ 9/9/9 N=36		-3.0			SP	D											11.7m: Grades to some sand. Low plasticity, slow. 12.0		
			100	SPT				-3.5															12.0m: Grades to sandy, minor gravel. Non-plastic, quick; gravel, fine to medium, sub-rounded. 12.5		
			100	SPT				-4.0															Fine to coarse, predominantly fine SAND, with some gravel and silt; grey. Dense, wet; gravel, fine to coarse, sub-rounded. 12.2m: Grades to gravel absent. Sand, fine. 12.5		
			100	SONIC				-4.5			SM												13.05m: Grades to silty, with minor interbedded silt lenses in sand, trace fibrous organics. 13.0		
			100	SPT		4/5/7/ 7/8/10 N=32		-5.0			SP	Sat											13.45m: 100mm SILT lens. 13.5		
			100	SONIC				-5.5															13.7m: Grades to interbedded silt lenses absent. Organics absent. Trace shell fragments. Some silt. Saturated. 14.0		
			100	SONIC				-5.5			SM												14.6m: Grades to silty. 14.5		

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH9-03
 Hole Location: See Attached Plan
 SHEET 1 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831366.53 mN 395688.4 mE	DRILL TYPE: AMS-17C	HOLE STARTED: 27/1/14
R.L.: 9.50 m	DRILL METHOD: SONIC	HOLE FINISHED: 27/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JXXM CHECKED: RRG

GEOLOGICAL		ENGINEERING DESCRIPTION																		
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)		COMPRESSIVE STRENGTH (MPa)		DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.	
														0	50	100	200			50
Topsoil								9.50	0.0		M	S							TOPSOIL: SILT, with minor sand and rootlets; brownish grey. Soft, moist; sand, fine.	
Springston Formation			67	SONIC				9.0	0.5		D	F							SILT, with some sand; brownish grey. Firm, dry; sand, fine. Trace iron staining. 0.4m: Grades to sandy. Trace fibrous organics. 0.5	
								8.5	1.0		SP	L							Fine SAND, with some silt; brownish grey with some iron staining. Loose, dry. 0.95m: Sand becomes fine to coarse. NO RECOVERY: 1.0-1.5m. 1.0	
			78	SPT		0/0/0 0/0/0 N=0		8.0	1.5		ML	W	VS						SILT, with some sand; grey. Very soft, wet, low plasticity, quick; sand, fine. 1.8m: Grades to trace sand, trace fibrous organics. Low to moderate plasticity, slow. NO RECOVERY: 1.85-1.95m. 2.0	
			100	SONIC				7.5	2.0		SM	L							2.25m: Grades to sandy, organics absent. Non-plastic, quick. 2.5	
			78	SPT		0/1/2/ 1/3/2 N=8		7.0	2.5		ML	S							Grades to silty, fine SAND; grey. Loose, wet. 3.0	
			91	SONIC				6.5	3.0		SM	L							Grades to silty, fine SAND; grey. Loose, wet. NO RECOVERY: 3.35-3.45m. 3.5	
			67	SPT		0/0/1/ 1/1/2 N=5		6.0	3.5		SM	L							Below 3.45m: Intermittent sandy SILT/silty SAND layers. Non-plastic, quick; sand, fine. 4.0	
								5.5	4.0											NO RECOVERY: 4.5-4.6m. 4.5
								5.0	4.5											Below 4.75m: Intermittent sandy SILT layers absent. NO RECOVERY: 4.9-5.0m.

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH9-03
 Hole Location: See Attached Plan
 SHEET 2 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831366.53 mN 395688.4 mE	DRILL TYPE: AMS-17C	HOLE STARTED: 27/1/14
R.L.: 9.50 m	DRILL METHOD: SONIC	HOLE FINISHED: 27/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JXXM CHECKED: RRG

GEOLOGICAL					ENGINEERING DESCRIPTION															
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE CONDITION	WEATHERING	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)		COMPRESSIVE STRENGTH (MPa)		DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
															0-25	25-100	100-200	0-50		
Springston Formation			100	SONIC		2/2/2/ 2/2/1 N=7		5.0	5.0	SM	W		L							Silty, fine SAND, with trace organics; grey. Loose, wet; organics, fibrous. NO RECOVERY: 5.0-5.05m.
			100	SONIC				4.0	5.5	SP										5.65m: Grades to some silt; sand, fine to medium.
			100	SPT				3.5	6.0	SM										5.85m: Grades to silty; sand, fine.
			100	SPT		0/0/0/ 0/1/1 N=2		3.0	6.5											Below 6.55m: Intermittent sandy SILT/silty SAND layers. Non-plastic, quick; sand, fine.
			100	SONIC				2.5	7.0	SM										Below 7.1m: Intermittent sandy SILT layers.
			100	SPT		0/0/0/ 0/2/2 N=4		2.0	7.5	ML			VS							SILT, with some sand and trace organics; grey. Very soft, wet, low plasticity, quick; sand, fine; organics, fibrous.
			100	SPT				1.5	8.0				F							8.05m: Grades to sand absent. Firm, moderate plasticity, non-dilatant.
			100	SONIC				1.0	8.5				St							8.45m: Becomes stiff.
			100	SPT				0.5	9.0				F							8.65m: Grades to minor fine sand. Firm, low plasticity, slow. 8.85m: Grades to sand absent. Low to moderate plasticity, very slow.
			100	SPT				9.5	9.5				S							9.55m: Becomes soft.

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH9-03
 Hole Location: See Attached Plan
 SHEET 3 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831366.53 mN 395688.4 mE	DRILL TYPE: AMS-17C	HOLE STARTED: 27/1/14
R.L.: 9.50 m	DRILL METHOD: SONIC	HOLE FINISHED: 27/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JXXM CHECKED: RRG

GEOLOGICAL				ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)		COMPRESSIVE STRENGTH (MPa)		DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
													0-25	25-50	50-100	100-250		
Springston Formation			91	SONIC		0/0/0/ 0/0/1 N=1		0.0		ML	W	S						SILT, with trace organics; grey. Soft, wet, low to moderate plasticity, very slow; organics, fibrous.
			78	SPT				10.5				VS						NO RECOVERY: 10.6-10.7m. 10.7m: Grades to sandy. Very soft, non-plastic, quick.
			100	SONIC				11.0		SM		MD						NO RECOVERY: 11.05-11.15m. Grades to silty, fine SAND; grey. Medium dense, wet. Intermittent thin silt lenses.
			56	SPT		1/3/7/ 8/9/7 N=31		11.5		SP		D						11.65m: Grades to some silt. Intermittent silt lenses absent.
			100	SONIC				12.0										12.35m: Grades to minor gravel. Becomes dense; gravel, fine to coarse, sub-rounded; sand, fine to coarse, predominantly fine. NO RECOVERY: 12.45-12.65m. 12.65m: Grades to gravel absent. Sand becomes fine. 12.75m: Grades to silty, trace shell fragments.
			100	SONIC				12.5		SM		S						Grades to sandy SILT, with intermittent thin sand lenses; grey. Soft, wet, non-plastic, quick; sand, fine. Trace shell fragments.
			100	SPT		0/0/0/ 0/1/0 N=1		13.0		ML		VS						13.45m: Grades to minor sand. Becomes very soft, low plasticity, slow. Intermittent sand lenses absent.
			95	SONIC				13.5										Silty, fine SAND; grey. Medium dense, wet. Intermittent silt lenses.
								14.0		SM		MD						14.55m: Grades to minor silt, intermittent silt lenses absent. Trace shell fragments. Sand, fine to coarse, predominantly fine to medium.
								14.5		SP								Sandy SILT; grey. Soft, wet, non-plastic, quick; sand, fine.
								15.0		ML		S						14.8m: Grades to sand absent. Low plasticity, slow.

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH9-03


Hole Location: See Attached Plan






















SHEET 4 OF 4















PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831366.53 mN 395688.4 mE	DRILL TYPE: AMS-17C	HOLE STARTED: 27/1/14
R.L.: 9.50 m	DRILL METHOD: SONIC	HOLE FINISHED: 27/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JXXM CHECKED: RRG

GEOLOGICAL						ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLER	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)			COMPRESSIVE STRENGTH (MPa)			DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
													0	25	50	50	100	200		
Springston Formation				100	SPT	3/4/5/ 5/9/10 N=29		5.5	X	SP	W	MD							Fine to medium SAND, with minor silt; grey. Medium dense, wet. 15.0m: 80mm SILT lens. 15.08m: Grades to some shell fragments. NO RECOVERY: 15.15-15.2m. 15.2m: Grades to silty. Trace shell fragments and fibrous organics; sand becomes fine.	
								6.0	X	SM									END OF BOREHOLE AT 15.65m. Target Depth Reached. Piezo Installed: 3m Screen 3m Blank	
								6.5												
								7.0												
								7.5												
								8.0												
								8.5												
								9.0												
								9.5												
								19.5												
								20												

T-T DATATEMPLATE.GDT.rtg

 www.aurecongroup.com	<h1 style="text-align: center;">BOREHOLE RECORD</h1>		HOLE NO. BH1
			PROJECT NO. 505601
PROJECT Ravenswood New World Bob Robertson Drive			
METHOD Borehole	CO-ORDINATES (NZTM)		SHEET 1 of 2
MACHINE & NO. Sonic	E 1573378 N 5205010		DATE from 28/03/2019 to 28/03/2019
FLUSHING MEDIUM Water	ORIENTATION VERTICAL		GROUND-LEVEL +11.00 m RL

Drilling Progress	Casing depth/size	Water level (m) shift start/end	Water Recovery %	Total core Recovery %	Solid core Recovery %	R.Q.D.	Fracture Index	Tests	Samples	Reduced Level	Depth (m)	Legend	STRATA DESCRIPTION
													<small>SUBORDINATE FRACTION, MAJOR FRACTION, MINOR FRACTION, COLOUR, STRUCTURE, STRENGTH, MOISTURE CONDITION GRADING, BEDDING, PLASTICITY, ETC... (NZ GEOTECHNICAL SOCIETY - FIELD DESCRIPTION OF SOIL AND ROCK)</small>
										0.00	0.00		
										+10.80	0.20		SILT with minor sand and trace rootlets; brown. Moist, non-plastic; sand, fine to medium. [TOPSOIL]
										+9.70	1.30		Sandy GRAVEL; brown. Moist, subangular to subrounded; sand, fine to coarse.
								(18, 21, 34, 26) N = 60/110 mm		+9.55	1.45		Fine to coarse GRAVEL with some sand; brown. Moist, subangular to subrounded; sand, fine to coarse. [FILL]
										+8.70	2.30		1.70m Becomes wet.
										+8.50	2.50		SILT with minor sand; light brown. <i>Stiff</i> , wet, low plasticity; sand, fine to medium.
										+8.20	2.80		Silty fine to coarse SAND; brown mottled orange. <i>Wet</i> .
								(1, 1, 1, 0, 1, 1) N = 3		+8.00	3.00		Fine to coarse SAND with some silt; brown. <i>Wet</i> .
										+7.00	4.00		SILT with minor sand; grey. <i>Wet</i> , low plasticity; sand, fine to coarse.
										+6.60	4.40		3.40m Becomes saturated.
										+6.60	4.40		3.50m Becomes with some sand, grey mottled orange and brown.
										+4.80	6.20		Fine to coarse SAND with minor silt; dark grey. Saturated.
								(7, 9, 8, 9, 9, 9) N = 35		+4.40	6.60		Fine to coarse GRAVEL with some sand; grey. Saturated, subangular to subrounded; sand, fine to coarse.
										+4.30	6.70		Sandy fine to coarse GRAVEL; grey. Saturated, subangular to subrounded; sand, fine to coarse.
								(8, 9, 8, 7, 5, 5) N = 25		+3.40	7.60		Fine to coarse SAND with minor gravel; grey. Saturated; gravel, fine to coarse, subangular to subrounded.
										+3.30	7.70		Sandy fine to coarse GRAVEL; grey. Saturated, subangular to subrounded; sand, fine to coarse.
								(7, 11, 6, 5, 5, 5) N = 21		+2.40	8.60		Fine to coarse SAND with minor gravel; grey. Saturated; gravel, fine to coarse, subangular to subrounded.
													8.50m Becomes with some silt.
													SILT with minor sand and trace roots; dark grey. Saturated, low plasticity; sand, fine to medium.
								(1, 1, 1, 0, 0, 3) N = 4					8.70m Becomes with no roots.
													9.50m - 9.55m Becomes with some organics; fibrous to woody.
													9.70m - 9.80m Becomes with some sand.

<ul style="list-style-type: none">  Small Disturbed Sample  Large Disturbed Sample  SPT Liner Sample  Thin Wall Undisturbed Sample  U100 Undisturbed Sample  Pocket Penetrometer Test  Piston Sample 	<ul style="list-style-type: none">  Water Level  Impression Packer Test  Standard Penetration Test  Permeability Test  Piezometer / Standpipe Tip  Packer Test  In-situ Vane Shear Test 	LOGGED S. MORRIS DATE 09/04/2019 CHECKED K. FOOTE DATE 10/04/2019	REMARKS Groundwater not recorded at time of drilling. Co-ordinates and elevation data from handheld GPS, accurate to +/-5m. Elevation based on CCC Drainage Datum. Auto-SPT Hammer with 93.0% efficiency.
--	---	--	--

Report ID: AGS4 BOREHOLE RECORD || Project: RAVENSWOOD NEW WORLD DCPS.GPJ || Library: AGS 4 0.GLB || Date: 12 November 2019

BOREHOLE RECORD

HOLE NO. **BH1**
PROJECT NO. **505601**

PROJECT **Ravenswood New World
Bob Robertson Drive**

METHOD **Borehole**
MACHINE & NO. **Sonic**

CO-ORDINATES (NZTM)
**E 1573378
N 5205010**

SHEET **2** of **2**
DATE from **28/03/2019** to **28/03/2019**

FLUSHING MEDIUM **Water**

ORIENTATION **VERTICAL**

GROUND-LEVEL **+11.00** m RL

Drilling Progress	Casing depth/size	Water level (m) shift start/end	Water Recovery %	Total core Recovery %	Solid core Recovery %	R.Q.D.	Fracture Index	Tests	Samples Type Ref Depth	Reduced Level	Depth (m)	Legend	STRATA DESCRIPTION
													SUBORDINATE FRACTION, MAJOR FRACTION, MINOR FRACTION, COLOUR, STRUCTURE, STRENGTH, MOISTURE CONDITION, GRADING, BEDDING, PLASTICITY, ETC. (NZ GEOTECHNICAL SOCIETY - FIELD DESCRIPTION OF SOIL AND ROCK)
			100							+0.80	10.20	x x x	Peaty SILT; black with grey inclusions. Saturated, non-plastic; fibrous with woody inclusions, no odour. SILT with minor sand; grey. Saturated, low plasticity; sand, fine. 11.20m - 11.30m Becomes with some sand. 11.60m Becomes with trace sand. 12.10m Becomes with no sand, trace clay. 12.80m Becomes with trace sand, no clay. 13.00m Becomes with minor sand. 14.00m - 14.20m Becomes with trace gravel, fine. 14.30m Becomes with some silt. 14.70m Becomes with minor silt. 15.10m Becomes with trace silt. 16.60m Becomes with minor silt.
			100					(2, 1, 2, 3, 2, 2) N=9		+0.60	10.40	x x x	
			100					(1, 0, 0, 4, 3, 0) N=7				x x x	
			100					(0, 0, 0, 0, 0, 0) N=0		-3.00	14.00	x x x	
			100					(7, 7, 8, 5, 2, 2) N=17				x x x	
			100					(7, 10, 11, 11, 13, 17) N=52	16.72	-5.72	16.72	x x x	
			100									x x x	
			100									x x x	
			100									x x x	
			100									x x x	
			100									x x x	
			100									x x x	
			100									x x x	

Report ID: AGS4 BOREHOLE RECORD | Project: RAVENSWOOD NEW WORLD DCPS GPJ | Library: AGS 4 0.GLB | Date: 12 November 2019

• Small Disturbed Sample	▼ Water Level
▬ Large Disturbed Sample	Impression Packer Test
▬ SPT Liner Sample	Standard Penetration Test
▬ Thin Wall Undisturbed Sample	Permeability Test
▬ U100 Undisturbed Sample	Piezometer / Standpipe Tip
▬ Pocket Penetrometer Test	Packer Test
▬ Piston Sample	In-situ Vane Shear Test

LOGGED **S. MORRIS**
DATE **09/04/2019**
CHECKED **K. FOOTE**
DATE **10/04/2019**

REMARKS
Groundwater not recorded at time of drilling.
Co-ordinates and elevation data from handheld GPS, accurate to +/-5m.
Elevation based on CCC Drainage Datum.
Auto-SPT Hammer with 93.0% efficiency.



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-01
 Hole Location: See Attached Plan
 SHEET 1 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831190.93 mN 395343.73 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.30 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL						ENGINEERING DESCRIPTION																	
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)			COMPRESSIVE STRENGTH (MPa)			DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.		
														0	25	100	50	100	200				
Topsoil																						Topsoil	
Springston Formation			100	SONIC				10.0		X	SM	M	L									Silty, fine SAND; light brown. Moist.	
								0.5		X	SP											0.6m: Grades to some silt. 0.8m: Grades to minor silt.	
			0	SPT			No SPT Barrel sunk	0.95		X													
								1.0		X													
								1.5		X	ML	M	F									Sandy SILT; light brown with orangey brown mottles. Moist, low plasticity.	
								1.5		X													NO RECOVERY: 1.5-1.95m.
								2.0		X	SM	W	VL										Silty SAND; brown. Loose, wet.
								2.0		X	SP												2.2m: Grades to minor silt. Becomes brown with orangey brown mottles; sand becomes fine to medium.
			100	SONIC				2.5		X													2.7m: Grades to some silt.
								3.0		X	ML		VS										Sandy SILT; greyish brown with orange mottles. Very soft, wet, low plasticity, quick; sand, fine.
						0/0/0 0/1/0 N=1	3.0		X														
							3.5		X			S										3.4m: Grades to trace sand. Becomes soft.	
							4.0		X													3.8m: Grades to trace organics. Becomes slow.	
		91	SONIC				4.5		X													NO RECOVERY: 4.5-4.6m.	
							4.5		X													4.6m to 4.7m: Minor wood pieces. 4.7m: Grades to trace rootlets.	
							5.0		X														

T-T DATATEMPLATE.GDT.rtg

Log Scale 1:25

BORELOG RAVENSWOOD 8-01,8-02,8-03&9.01.GPJ 25-Mar-2014



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-01
 Hole Location: See Attached Plan
 SHEET 2 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831190.93 mN 395343.73 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.30 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL										ENGINEERING DESCRIPTION															
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)					COMPRESSIVE STRENGTH (MPa)					DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
														0	25	50	100	200	50	100	200	500	1000		
Springston Formation			100	SONIC					5.0	X	ML	W	S										SILT, with trace sand, rootlets and organics; greyish brown with orange mottles. Soft, wet, low plasticity, slow; sand, fine.		
								5.5	X	X	SM		L										Silty SAND; grey. Wet. 5.4m to 5.5m: Wood pieces		
						2/1/0/ 1/1/1 N=3		4.0	X	X	ML		S										6.1m: Grades to some sand. Becomes quick.		
								6.5	X	X	SM		VL										NO RECOVERY: 6.4-6.55m.		
								7.0	X	X	SM		SP										Silty SAND; grey. Wet. 6.75m: Grades to minor silt.		
				76	SONIC				7.5	X	X													NO RECOVERY: 7.35-7.6m.	
						No SPT Barrel sunk		8.0	X	X	SM													7.7m: Grades to silty. Becomes very soft.	
								8.5	X	X	ML			S										SILT, with trace sand; grey. Soft, wet, low plasticity, slow; sand, fine. 8.5m: Becomes non-dilatant.	
								9.0	X	X	ML													NO RECOVERY: 8.75-9.1m.	
						0/1/1/ 1/1/2 N=5		1.0	X	X	SM			L										Silty, fine SAND; grey. Loose, wet.	
							9.5	X	X	SP													NO RECOVERY: 9.4-9.55m 9.4m: Grades to trace silt. Sand becomes fine to coarse.		
			91	SONIC				9.5	X	X													9.85m: Grades to some silt. Sand becomes fine.		

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-01
 Hole Location: See Attached Plan
 SHEET 3 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831190.93 mN 395343.73 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.30 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL					ENGINEERING DESCRIPTION																
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)			COMPRESSIVE STRENGTH (MPa)			DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
														0	25	50	50	100	200		
Springston Formation			91	SONIC		1/1/2/ 2/2/3 N=9		0.0	10.5		SM	W	L								Silty, fine SAND; grey. Loose, wet.
			100	SPT				-0.5	11.0												10.5 to 10.6m: Trace organics. NO RECOVERY: 10.6-10.7m.
			100	SONIC				-1.0	11.5												
			100	SONIC				-1.5	12.0												
			100	SPT		4/8/8/ 10/12/12 N=42		-2.0	12.5		SW		D								12.1m: Grades to trace gravel. Gravel is medium. Fine to coarse GRAVEL, with some sand; grey. Dense, wet; gravel, sub-angular to sub-rounded; sand, fine to coarse.
			100	SONIC				-2.5	13.0												13.0m: Grades to some sand.
			100	SONIC				-3.0	13.5		ML		F								SILT, with trace sand; grey. Firm, wet, low plasticity, slow; sand, fine. 13.3m: Grades to trace black organics.
			100	SPT		1/2/1/ 2/1/1 N=5		-3.5	14.0		SM		L								13.5 to 13.6m: Fine to coarse SAND lens. 13.6m: Grades to sandy. Silty, fine SAND; grey. Loose, wet.
			100	SONIC				-4.0	14.5		SP										14.3m: Grades to some silt. Sand becomes fine to coarse.
								-4.5	15.0												

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-01
 Hole Location: See Attached Plan
 SHEET 4 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831190.93 mN 395343.73 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.30 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL						ENGINEERING DESCRIPTION															
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)			COMPRESSIVE STRENGTH (MPa)			DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
														0	25	50	50	100	250		
Springston Formation			100	SONIC		1/1/1/ 1/1/1 N=4		-5.0		X	SP	W	L								Fine to coarse SAND, with trace silt and brown organics; grey. Loose, wet. 15.2m: Grades to some silt. 15.4m: Grades to silty. Sand becomes fine.
			100	SPT				15.5		X	SM										END OF BOREHOLE AT 15.65m. Target Depth Reached. Piezo Installed: 3m Screen 3m Blank
								-5.5													
								16.0													
								-6.0													
								16.5													
								-6.5													
								17.0													
								-7.0													
								17.5													
								-7.5													
								18.0													
								-8.0													
								18.5													
								-8.5													
								19.0													
								-9.0													
								19.5													
								-9.5													
								20													

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-02

Hole Location: See Attached Plan

SHEET 1 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831179.19 mN 395569.23 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.10 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.										SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.									
FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)	COMPRESSIVE STRENGTH (MPa)	DEFECT SPACING (mm)				
							10.0			ML	M	S							
										SM									
		87	SONIC				9.5												
							9.0												
							1.5			ML									
					No SPT Barrel sunk		8.5												
							2.0			SM	W	VL							
		87	SONIC				8.0												
							7.5			ML		S							
							3.0					F							
					0/1/0/ 1/1/2 N=4		7.0					VS							
		100	SPT				6.5					S							
							3.5					F							
							4.0												
		100	SONIC				6.0												
							4.5					S							
					0/0/0/ 1/1/0 N=2		5.5												
		100	SPT				5.0			SM		VL							

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-02
 Hole Location: See Attached Plan
 SHEET 2 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831179.19 mN 395569.23 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.10 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL						ENGINEERING DESCRIPTION															
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)			COMPRESSIVE STRENGTH (MPa)			DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
														0	25	50	100	200	50		
Springston Formation			100	SONIC		0/0/1/ 1/3/5 N=10		5.0		SM	W	VL								Silty, fine SAND; light brown. Very loose, wet.	
			100	SONIC				5.5													
			100	SPT				4.5		ML		S								Sandy SILT; grey. Soft, wet, low plasticity, quick; sand, fine.	
			100	SPT				6.0													
			100	SPT				4.0		SM		MD								Silty, fine SAND; grey. Medium dense, wet.	
			86	SONIC				3.5		SP										6.5m: Grades to trace silt.	
			78	SPT				7.0												6.9m: Grades to some silt.	
			78	SPT				2.5		SM										7.2m: Grades to silty.	
			100	SONIC				7.5		SP										7.4m: Grades to minor silt. NO RECOVERY: 7.45 - 7.6m.	
			100	SONIC				8.0												7.6m: Grades to some silt. NO RECOVERY: 7.95-8.05m.	
		78	SONIC				2.0		ML		S								SILT, with some sand and trace organics; grey with black organics. Soft, wet, low plasticity, slow; sand, fine.		
		100	SPT				8.5														
		100	SPT				9.0														
		78	SONIC				1.0														
		100	SPT				9.5					F							9.5m: Becomes firm.		
		78	SONIC				0.5														

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-02
 Hole Location: See Attached Plan
 SHEET 3 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831179.19 mN 395569.23 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.10 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.										SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.									
FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)	COMPRESSIVE STRENGTH (MPa)	DEFECT SPACING (mm)				
Springston Formation										<p>SILT, with some sand and trace organics; grey with black organics. Soft, wet, low plasticity, slow; sand, fine.</p> <p>SILT, with some sand and trace organics; grey with black organics. Firm, wet, low plasticity, slow; sand, fine.</p> <p>NO RECOVERY: 10.45-10.7m. 10.5</p> <p>11.0m: Becomes firm. 11.0</p> <p>Silty, fine SAND; grey. Very loose, wet. 12.0</p> <p>13.0m: Grades to trace black and brown organics. 13.0</p> <p>13.1m: Grades to minor silt.</p> <p>13.3m: Grades to silty.</p> <p>Fine to coarse GRAVEL, with minor sand and trace silt; grey. Medium dense, wet; sand, fine to coarse. 13.5</p> <p>NO RECOVERY: 13.55-13.7m.</p> <p>Silty, fine SAND; grey. Medium dense, wet.</p> <p>NO RECOVERY: 14.0-14.15m. 14.0</p> <p>14.5m: Grades to trace shells. 14.5</p>									
		78	SONIC		0/1/0/ 1/2/0 N=3		0.0		X	ML	W	S							
		100	SPT				10.5		X			F							
		100	SONIC				11.0		X										
		100	SONIC				11.5		X										
		100	SPT		0/0/1/ 1/1/0 N=3		12.0		X	SM		VL							
		100	SPT				12.5		X										
		86	SONIC				13.0		X	SP									
							13.5		X	SM									
							14.0		X	GW		MD							
		67	SPT		3/3/3/ 4/3/4 N=14		14.5		X	SM									
		100	SONIC				15.0		X										

T-T DATATEMPLATE.GDT.rtg



TONKIN & TAYLOR LTD

BOREHOLE LOG

BOREHOLE No: BH8-02

Hole Location: See Attached Plan

SHEET 4 OF 4

PROJECT: Ravenswood Subdivision	LOCATION: Ravenswood, Woodend	JOB No: 53214.100
CO-ORDINATES: 831179.19 mN 395569.23 mE	DRILL TYPE: Mobile drill	HOLE STARTED: 24/1/14
R.L.: 10.10 m	DRILL METHOD: SONIC	HOLE FINISHED: 24/1/14
DATUM: Mount Pleasant (2000)	DRILL FLUID: Drill pro	DRILLED BY: Prodrill Ltd
		LOGGED BY: JIMB CHECKED: RRG

GEOLOGICAL						ENGINEERING DESCRIPTION															
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION.	FLUID LOSS	WATER	CORE RECOVERY (%)	METHOD	CASING	TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)			COMPRESSIVE STRENGTH (MPa)			DEFECT SPACING (mm)	SOIL DESCRIPTION Soil type, minor components, plasticity or particle size, colour. ROCK DESCRIPTION Substance: Rock type, particle size, colour, minor components. Defects: Type, inclination, thickness, roughness, filling.
														0	25	50	50	100	250		
Springston Formation			100	SONIC		5/8/8/ 8/11/11 N=38		-5.0			SW	W	MD								Fine to coarse SAND, with minor silt; grey. Medium dense, wet.
			100	SPT				15.5			SM		D								15.2m: Grades to silty with trace shells. Becomes dense.
								-5.5													END OF BOREHOLE AT 15.65m. Target Depth Reached.
								16.0													Piezo Installed: 3m Screen 3m Blank
								-6.0													
								16.5													
								-6.5													
								17.0													
								-7.0													
								17.5													
								-7.5													
								18.0													
								-8.0													
								18.5													
								-8.5													
								19.0													
								-9.0													
								19.5													
								-9.5													
								20													

T-T DATATEMPLATE.GDT.rtg



CPT_126065

CPT_126066

CPT_126060

CPT_126062

CPT_126063

CPT_201450



CPT_37926

CPT_37927

CPT_37929

CPT_37930

CPT_37505

CPT_37919

CPT_37920

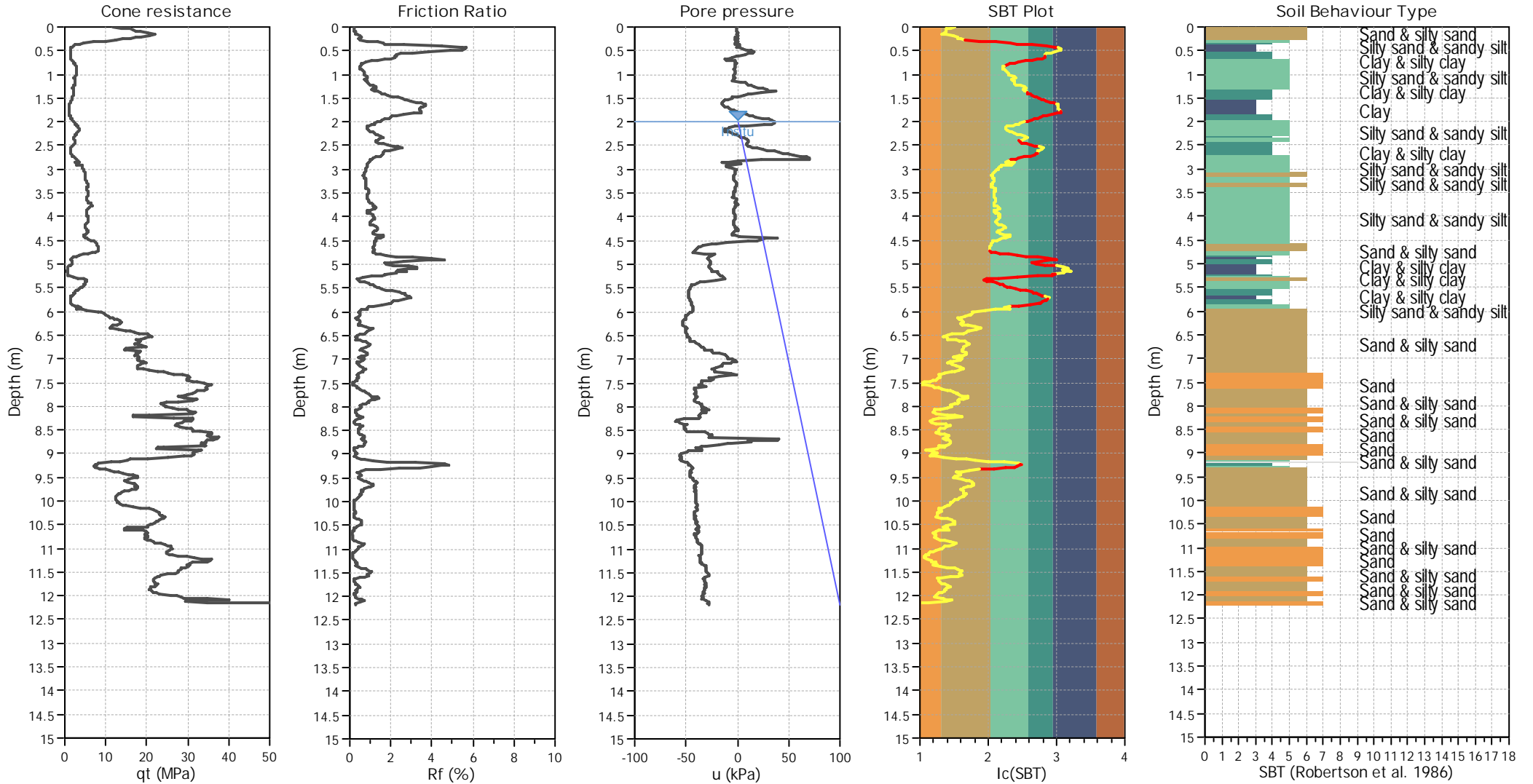
CPT_37923

CPT_37922

CPT_379

Appendix E. CLIQ Analysis

CPT basic interpretation plots



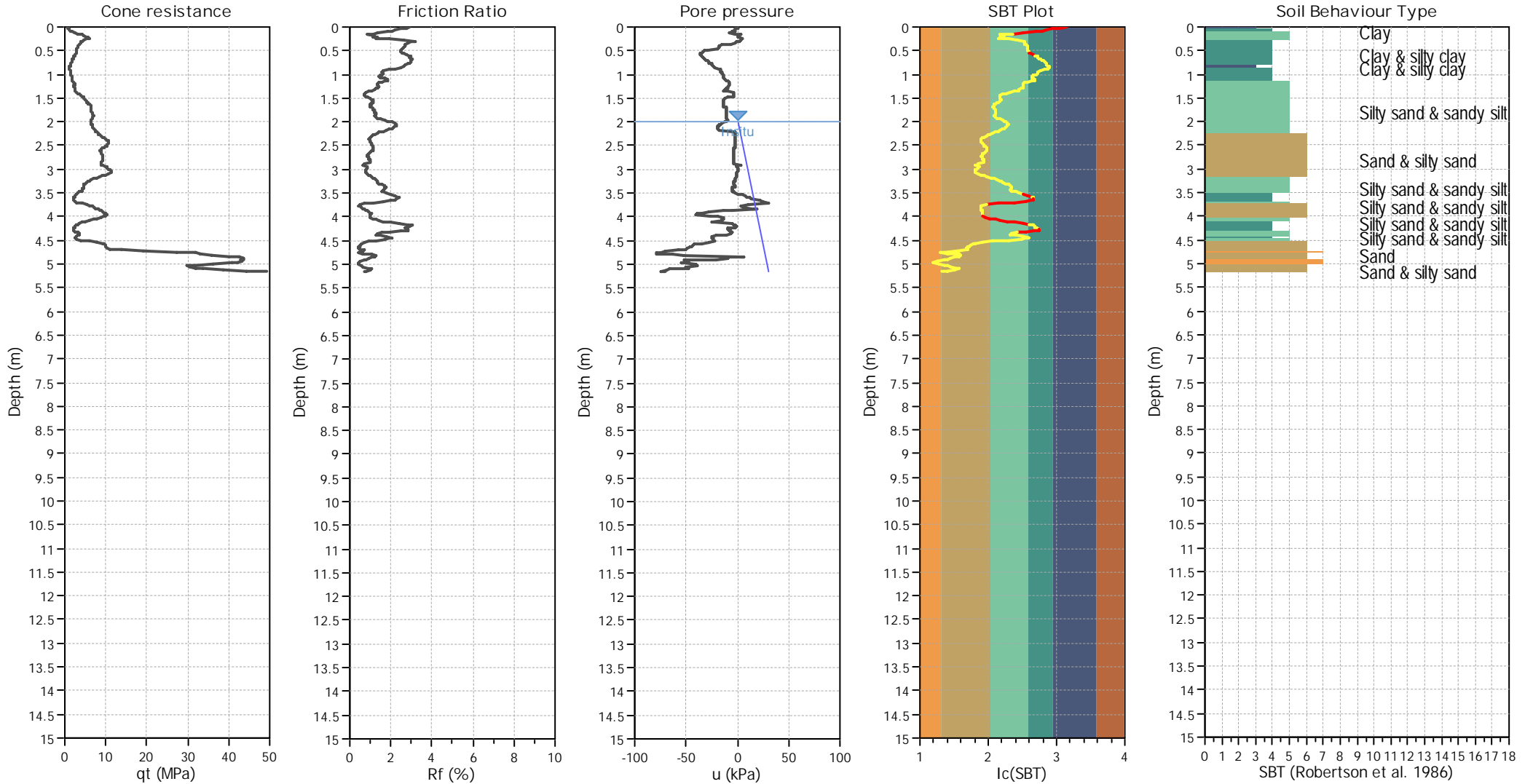
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots



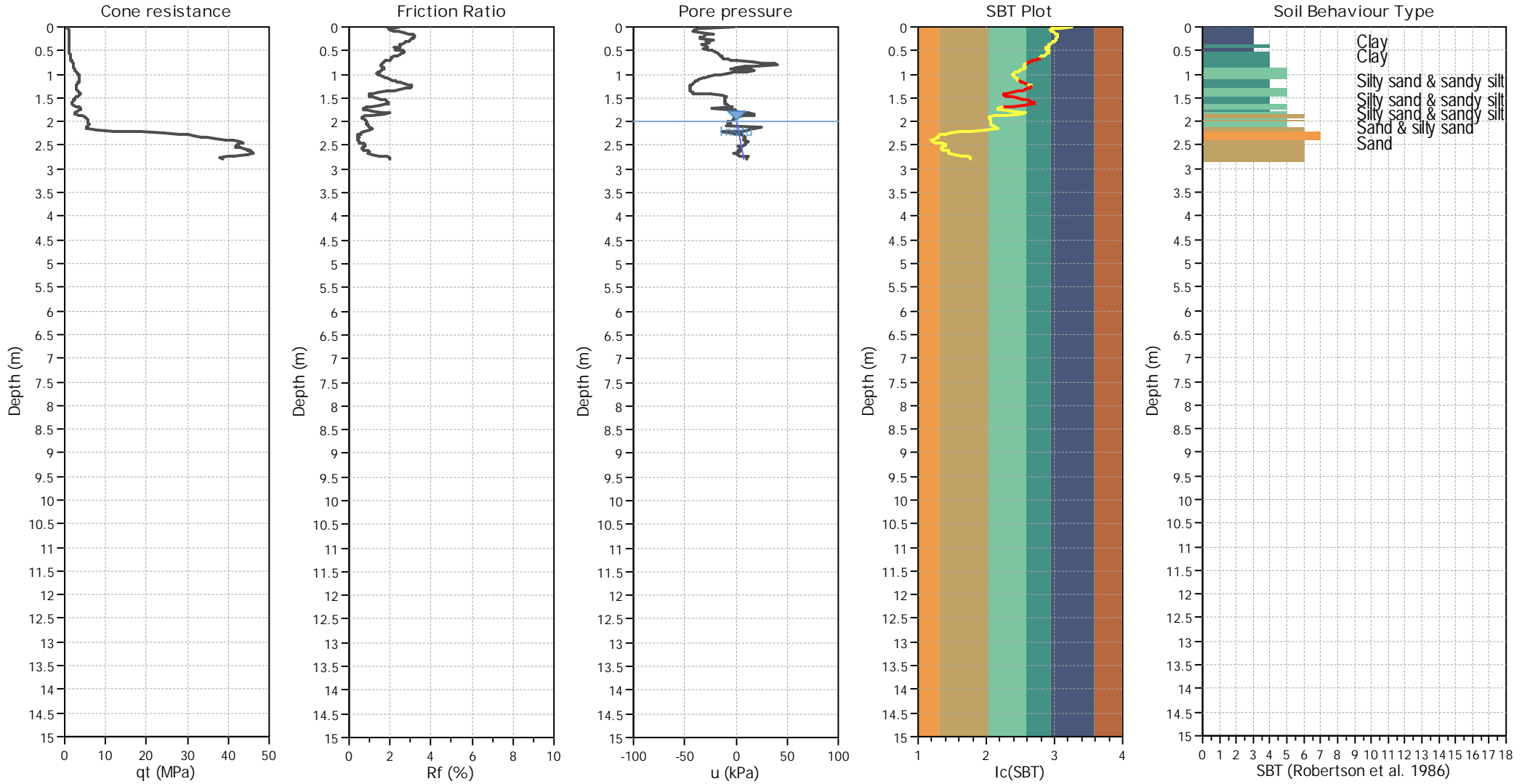
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_g applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots



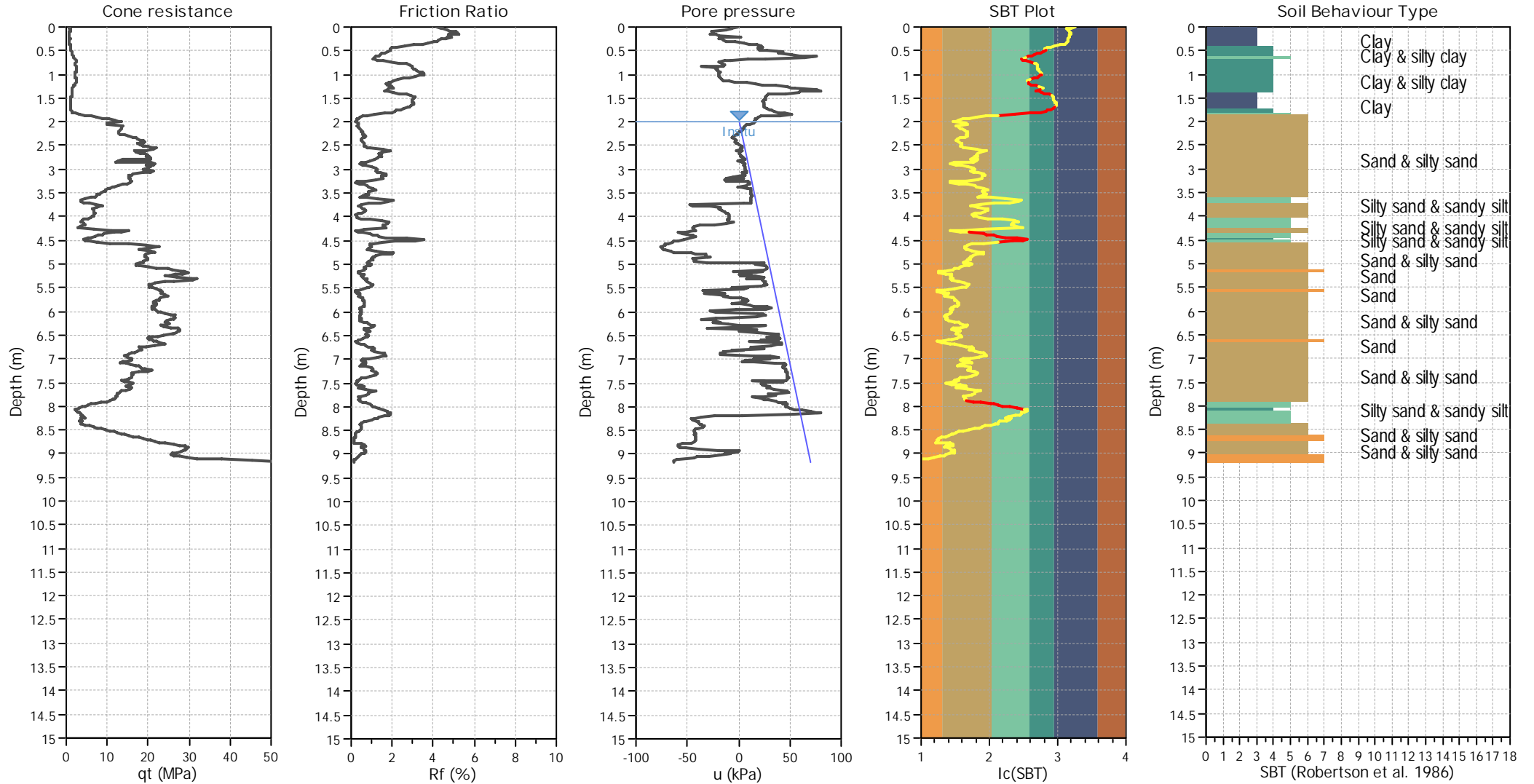
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_g applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

■ 1. Sensitive fine grained	■ 4. Clayey silt to silty	■ 7. Gravely sand to sand
■ 2. Organic material	■ 5. Silty sand to sandy silt	■ 8. Very stiff sand to
■ 3. Clay to silty clay	■ 6. Clean sand to silty sand	■ 9. Very stiff fine grained

CPT basic interpretation plots



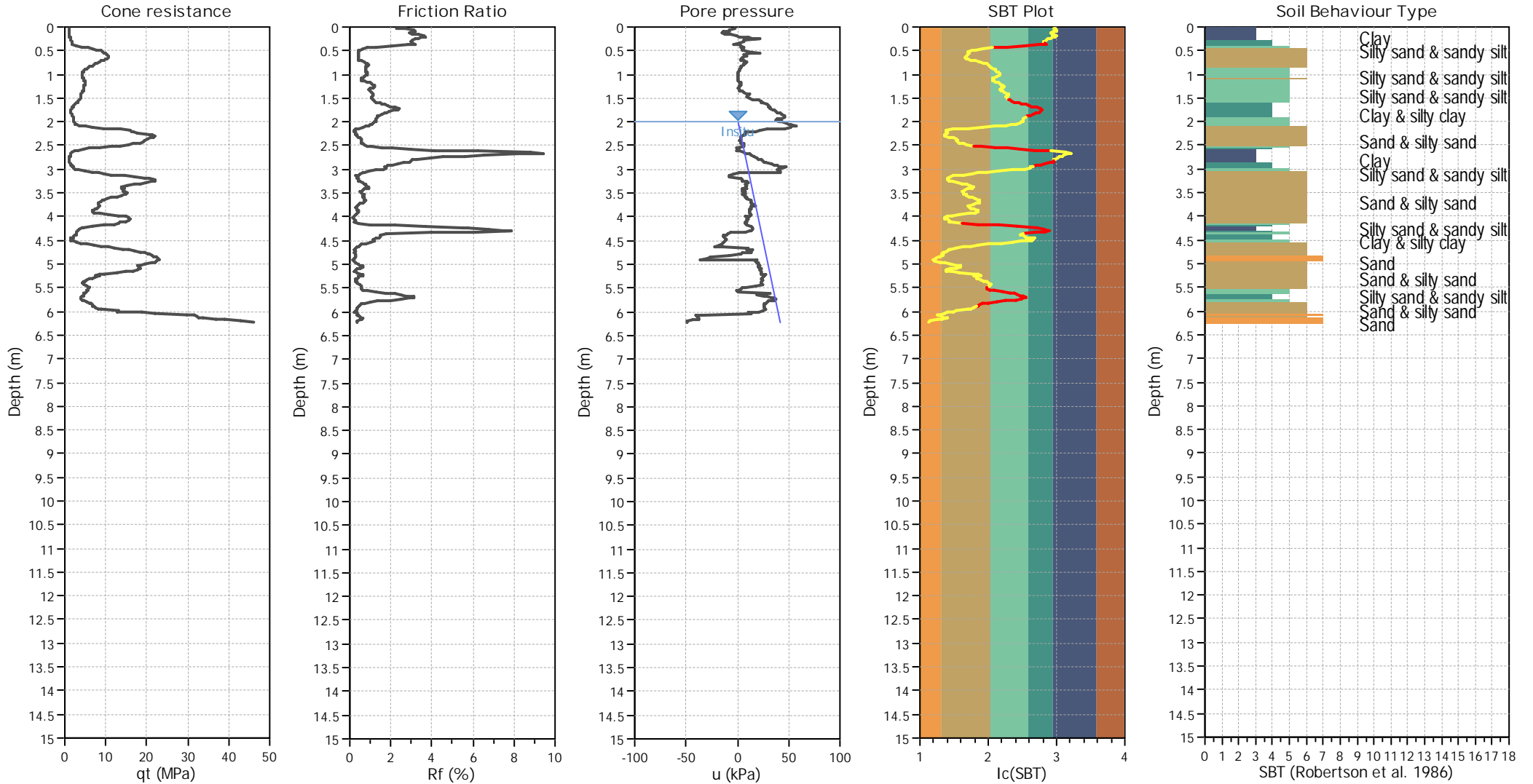
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

■ 1. Sensitive fine grained	■ 4. Clayey silt to silty	■ 7. Gravely sand to sand
■ 2. Organic material	■ 5. Silty sand to sandy silt	■ 8. Very stiff sand to
■ 3. Clay to silty clay	■ 6. Clean sand to silty sand	■ 9. Very stiff fine grained

CPT basic interpretation plots



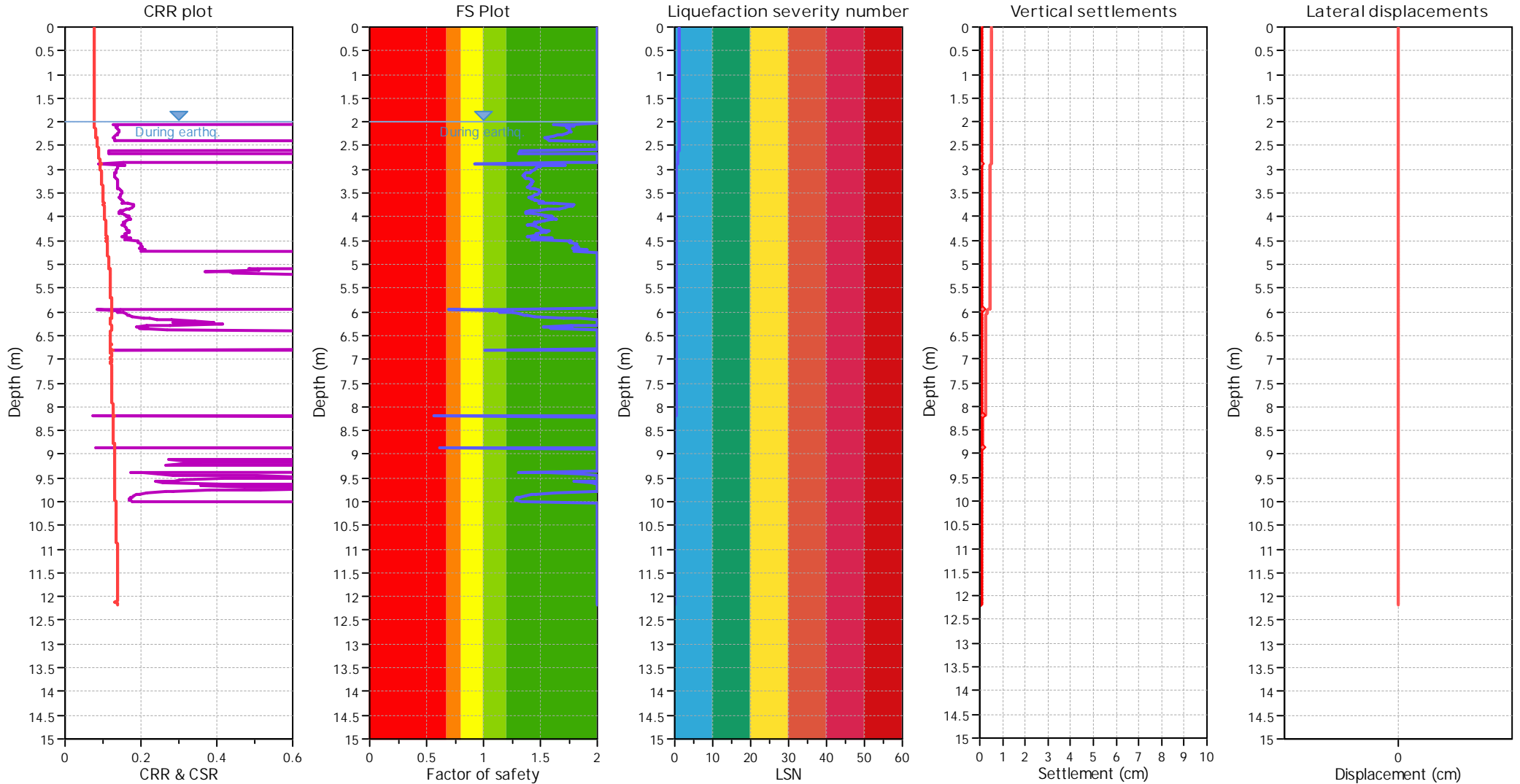
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_g applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

■ 1. Sensitive fine grained	■ 4. Clayey silt to silty	■ 7. Gravely sand to sand
■ 2. Organic material	■ 5. Silty sand to sandy silt	■ 8. Very stiff sand to
■ 3. Clay to silty clay	■ 6. Clean sand to silty sand	■ 9. Very stiff fine grained

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

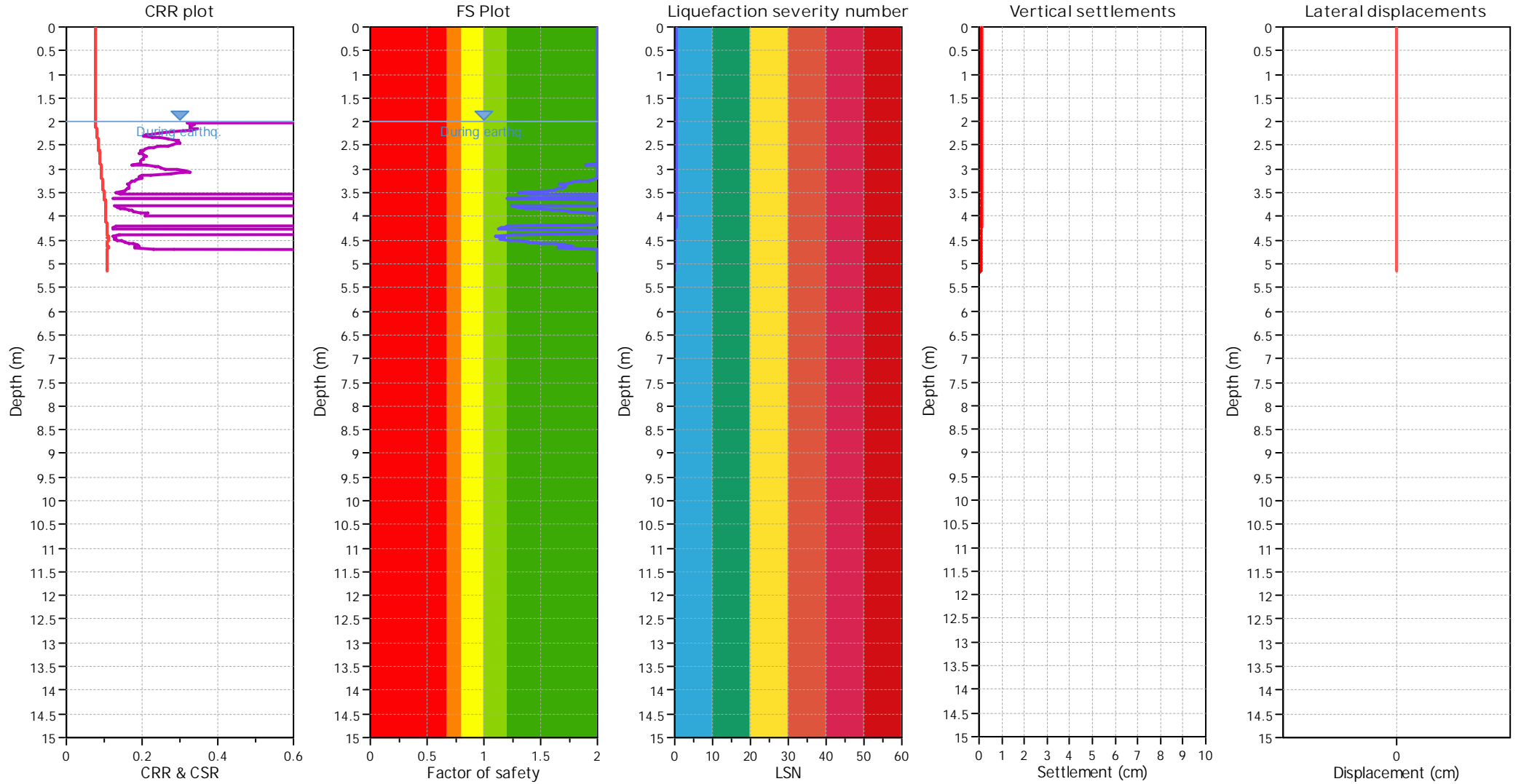
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_f applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

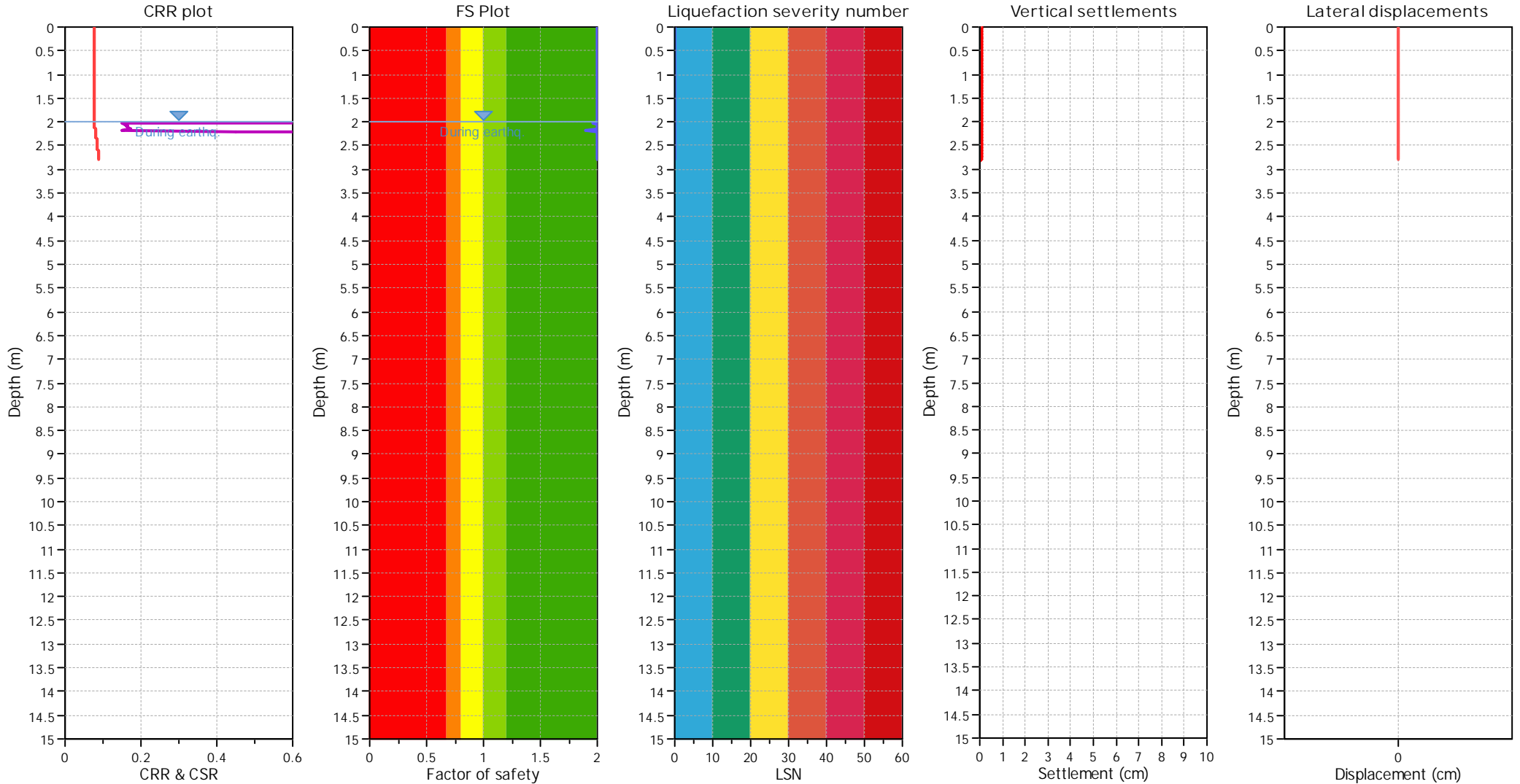
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

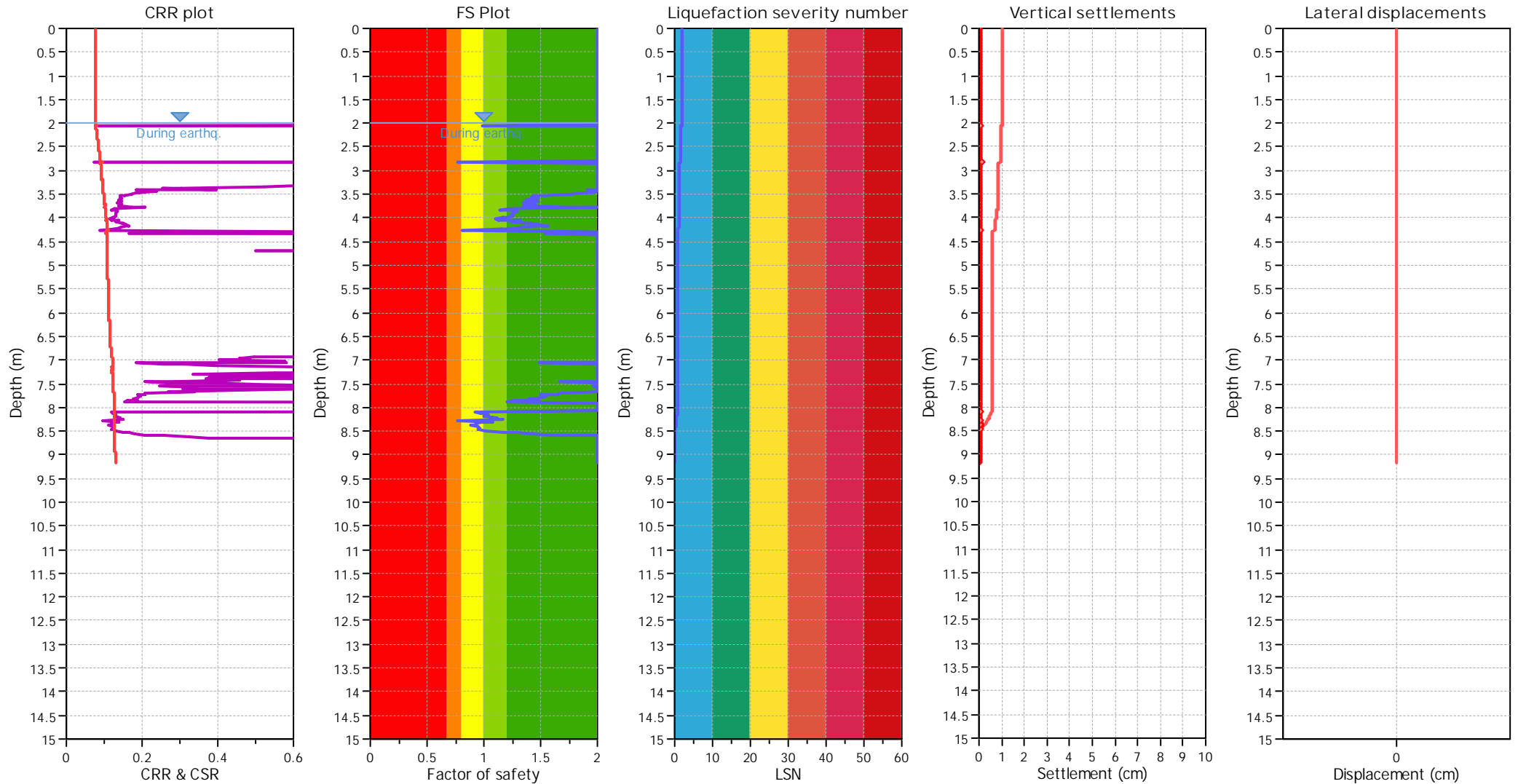
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

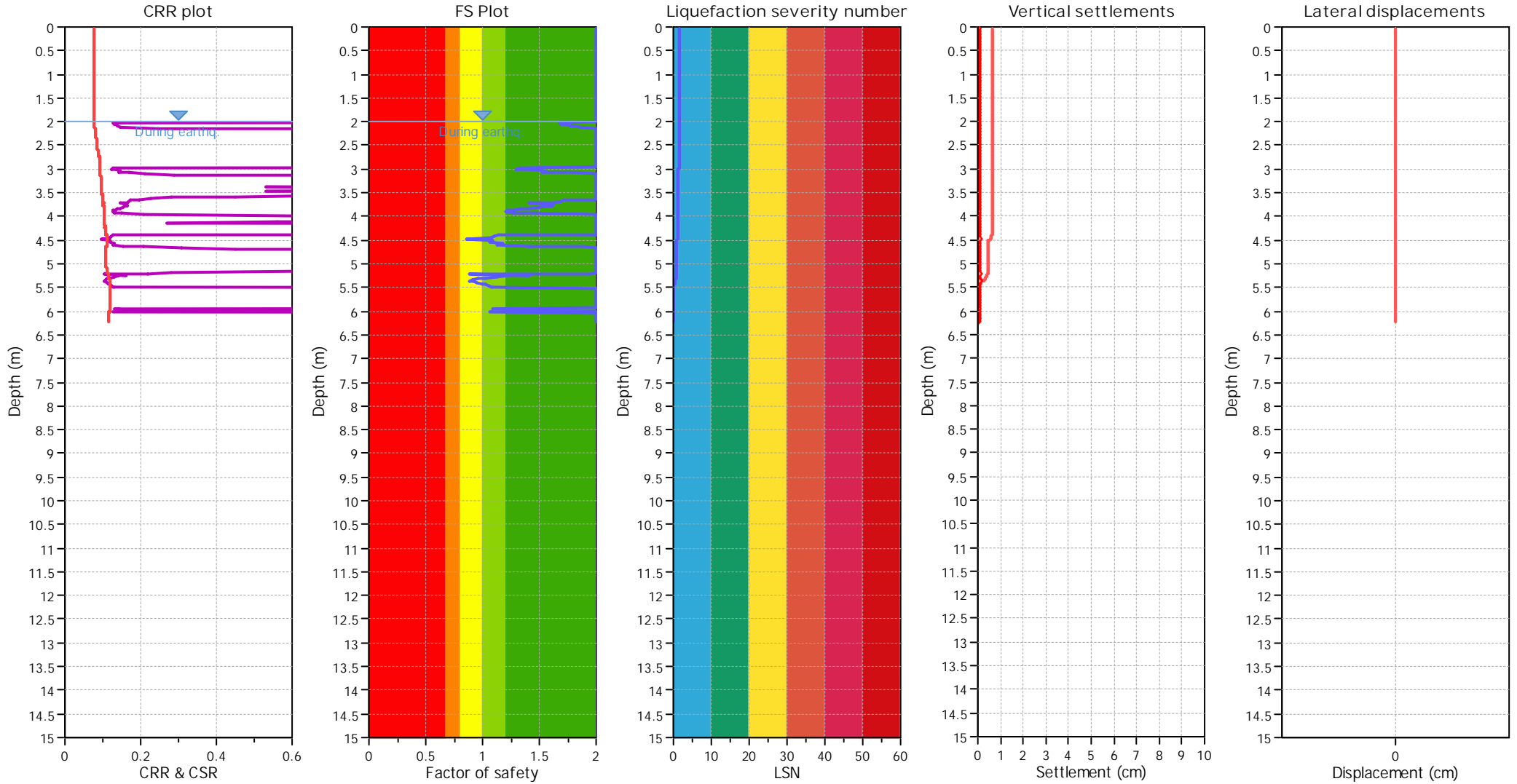
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

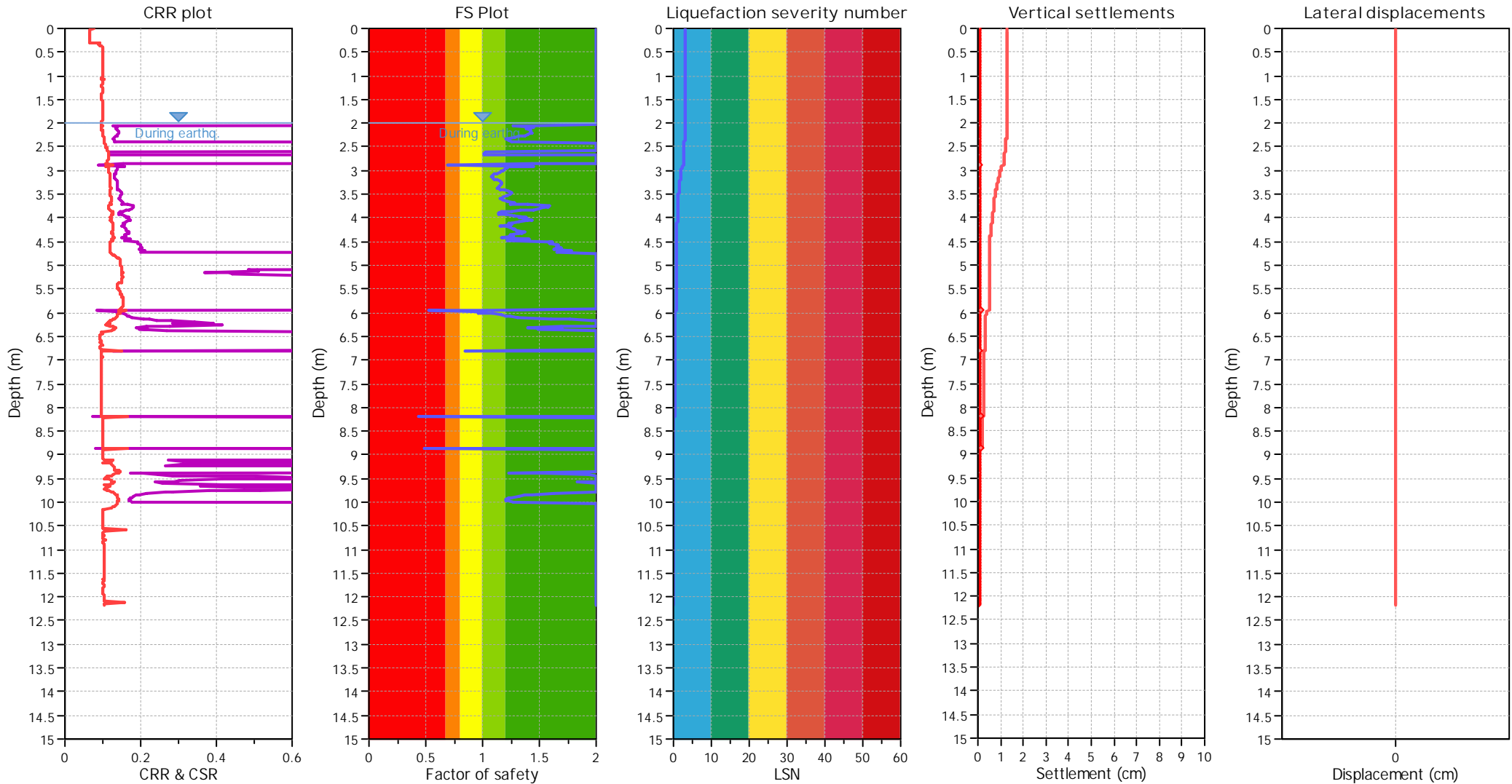
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

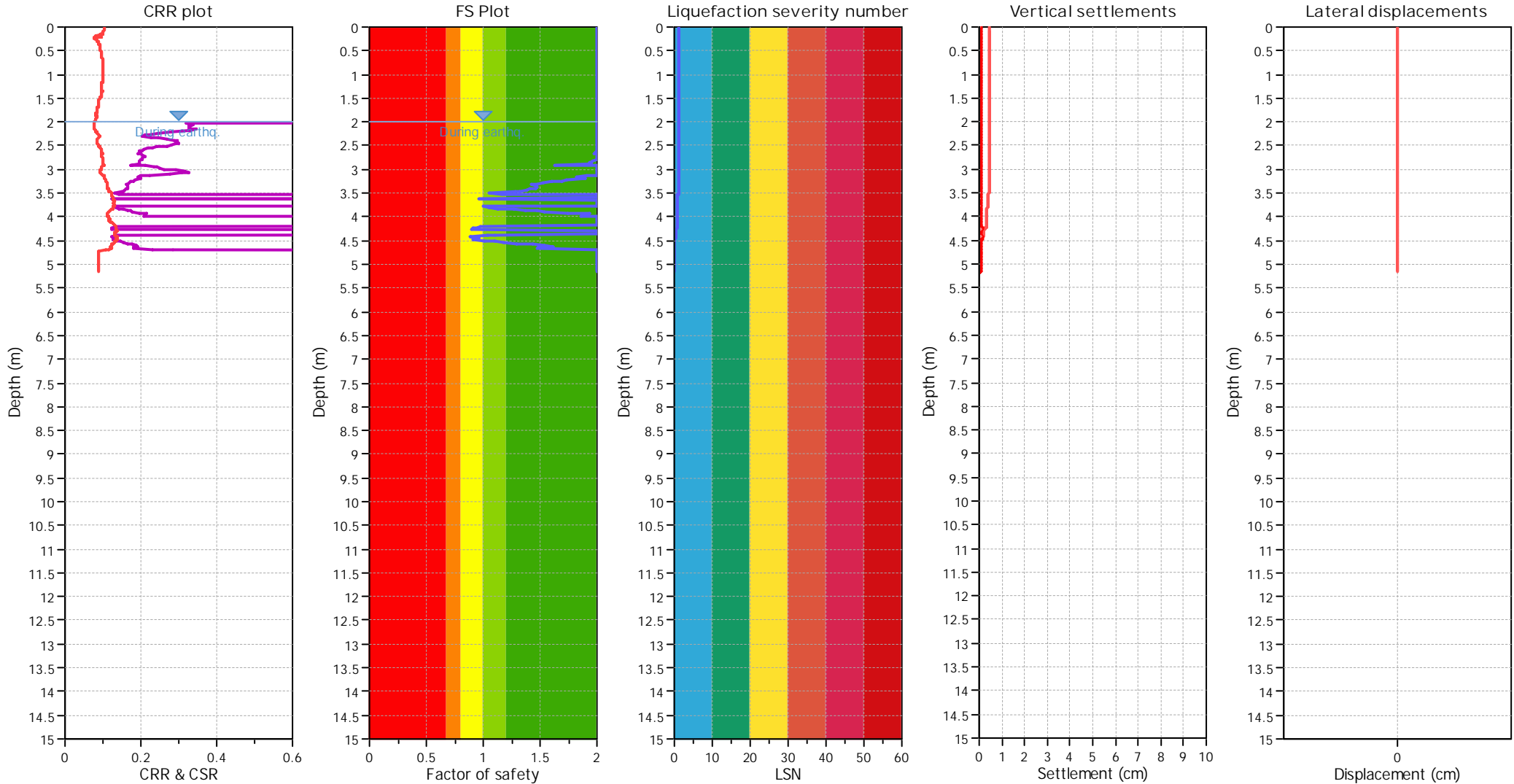
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

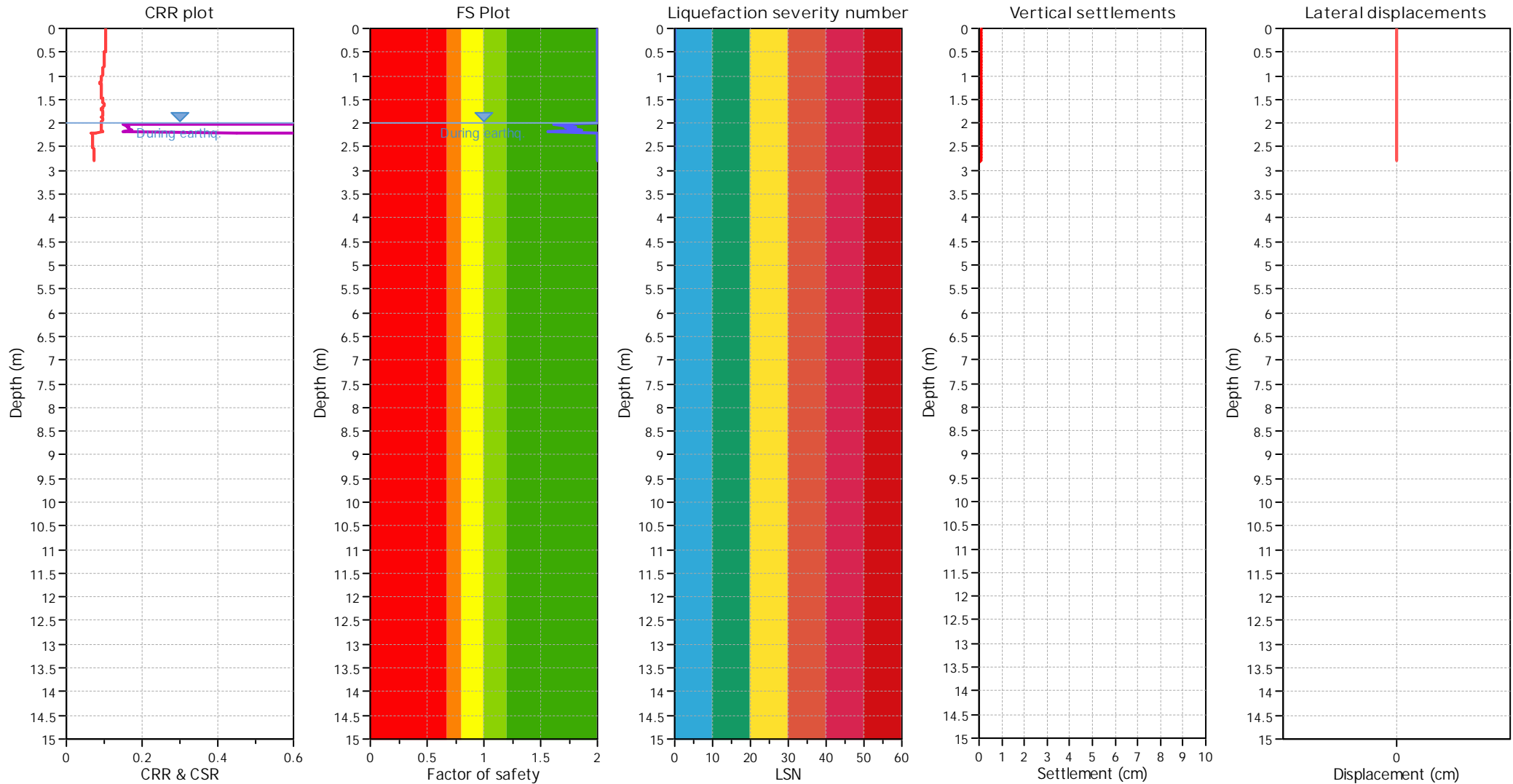
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_d applied:	Yes
Earthquake magnitude M_w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

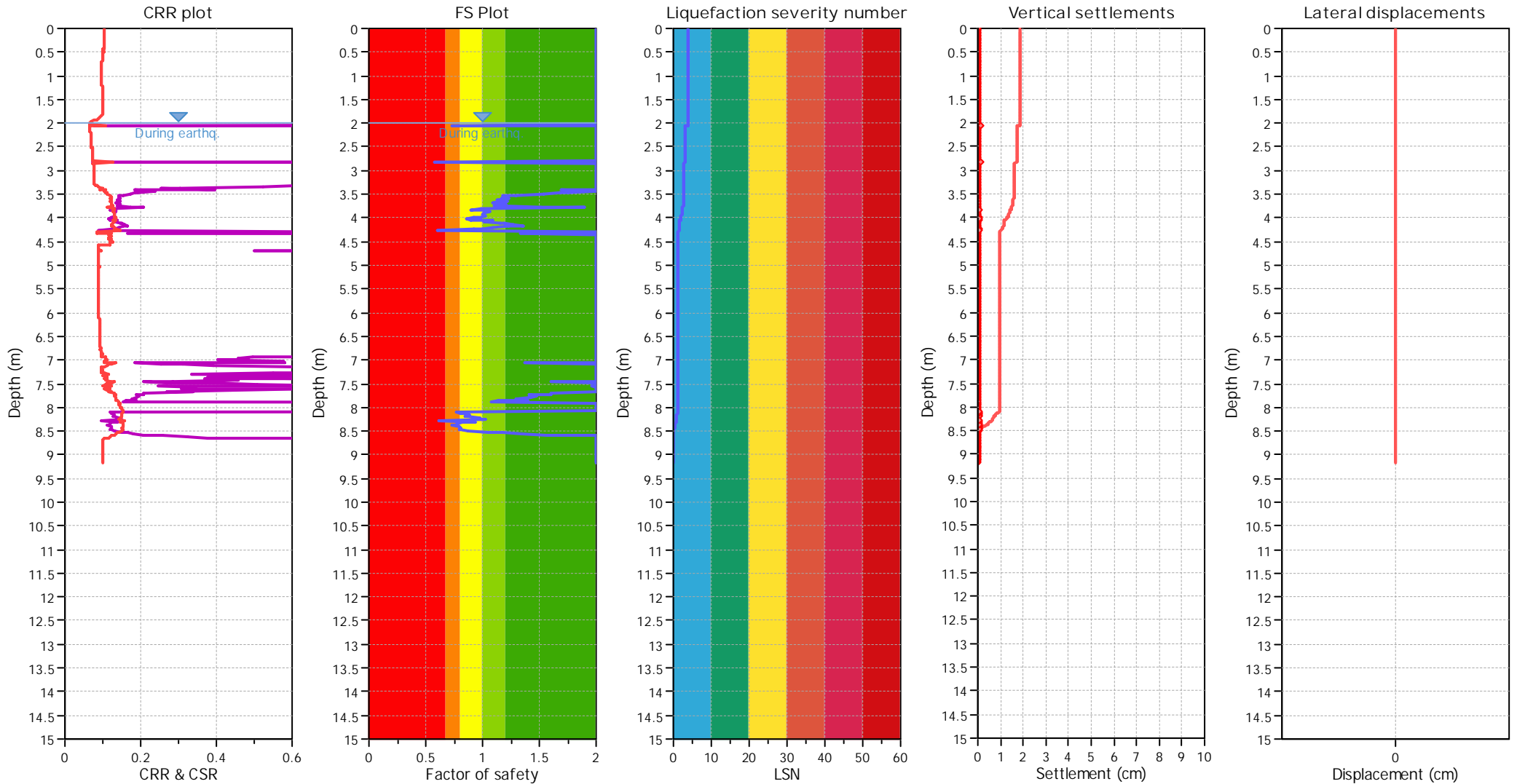
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on I _c value	I _c cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

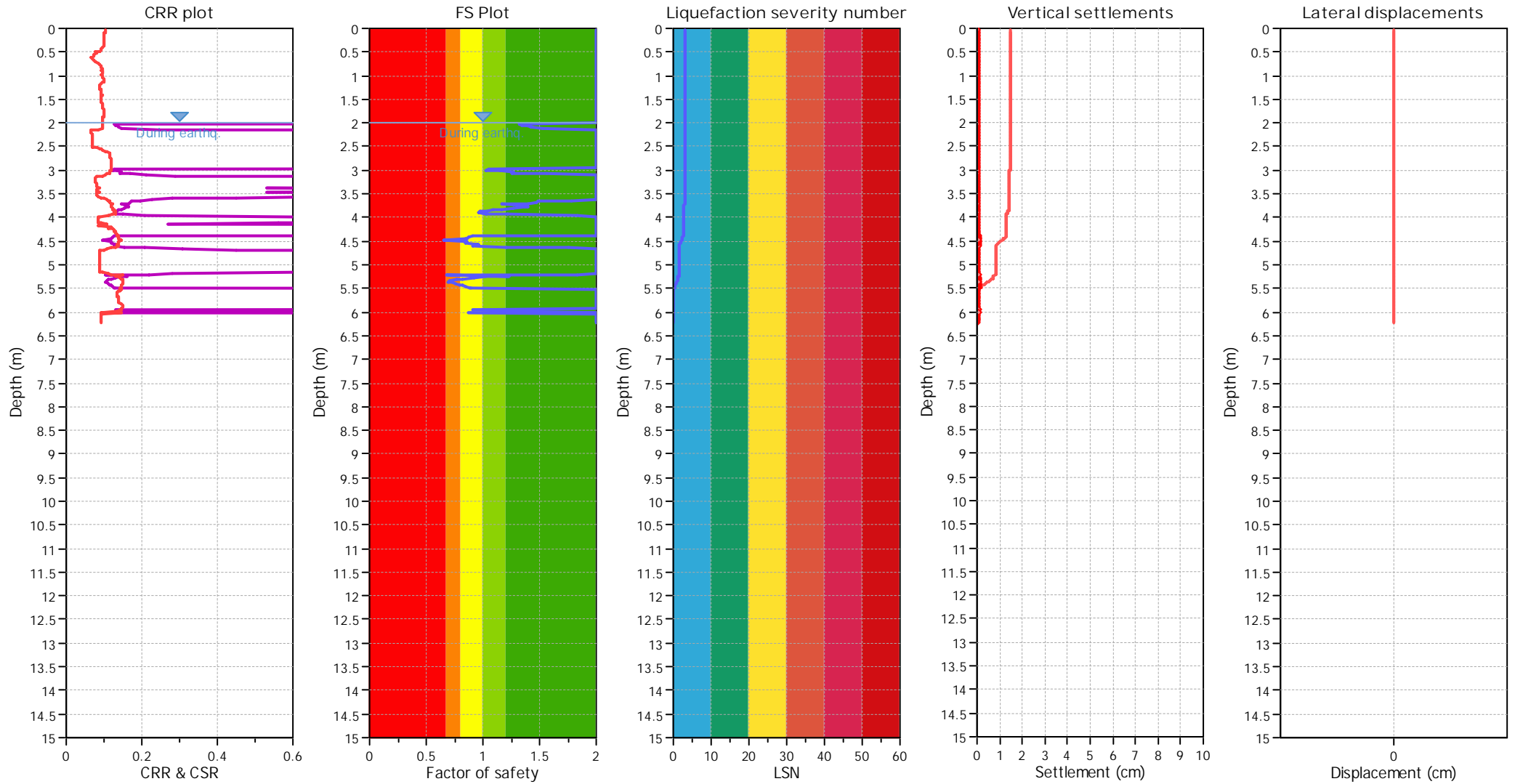
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

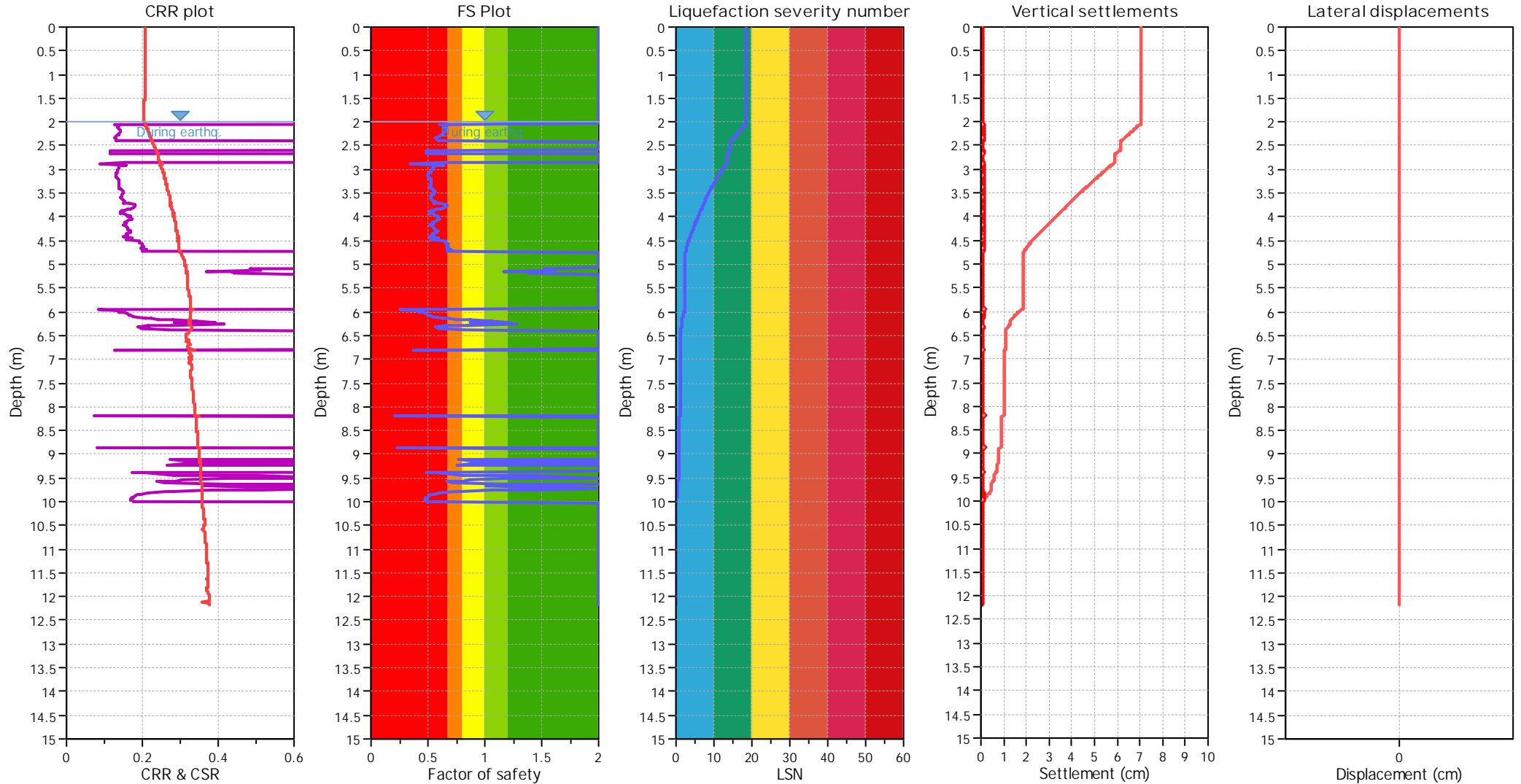
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

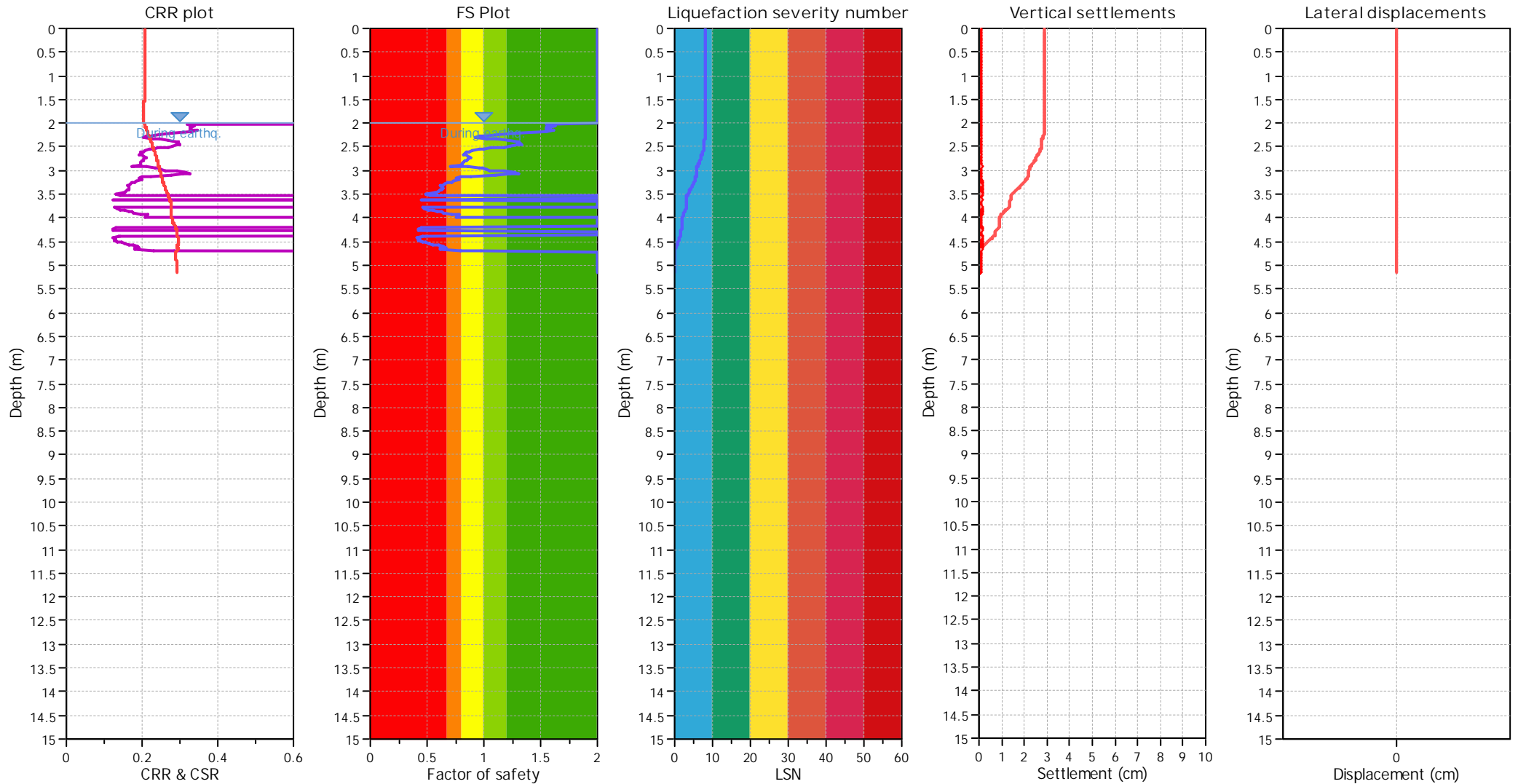
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on I _c value	I _c cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

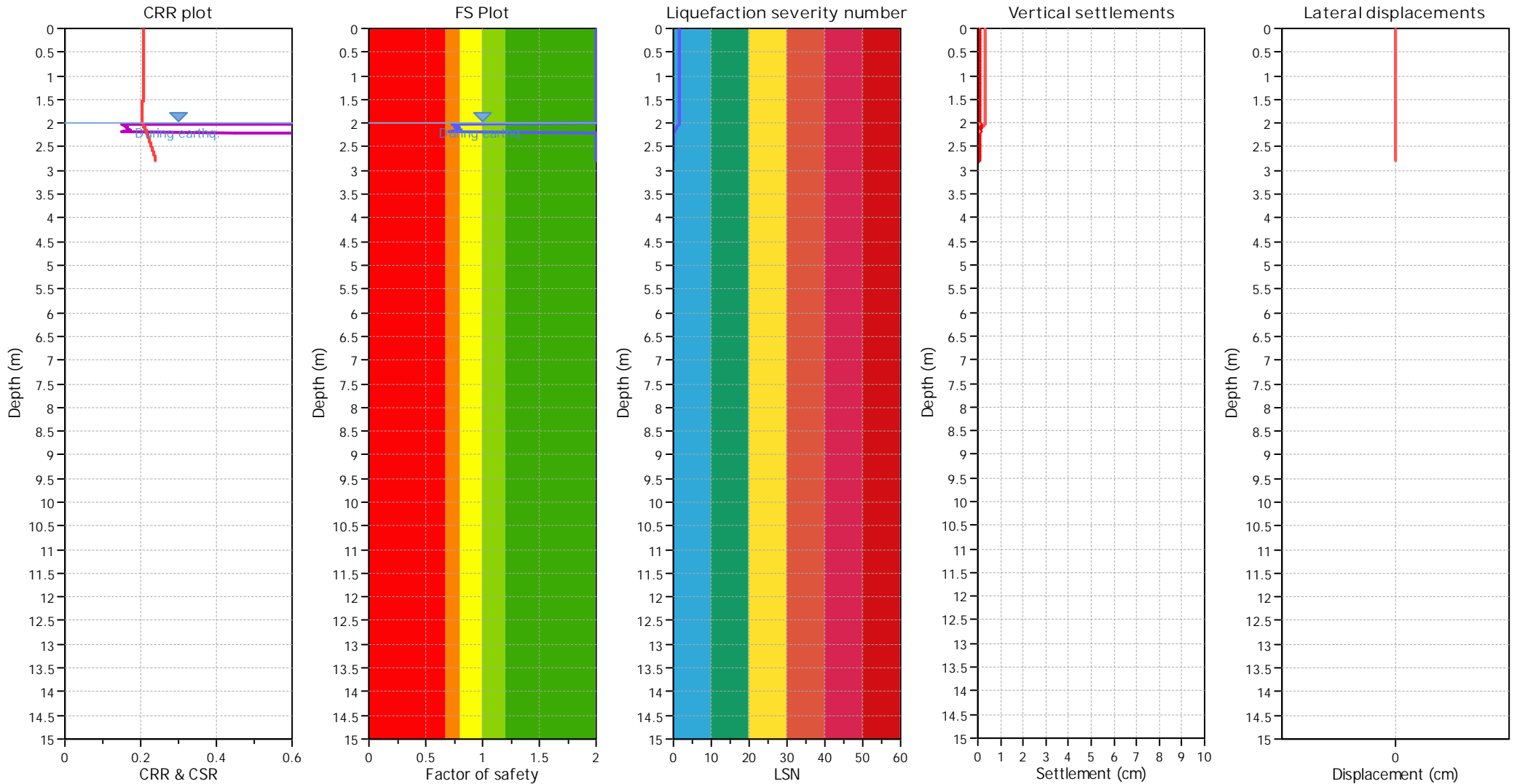
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_0 applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

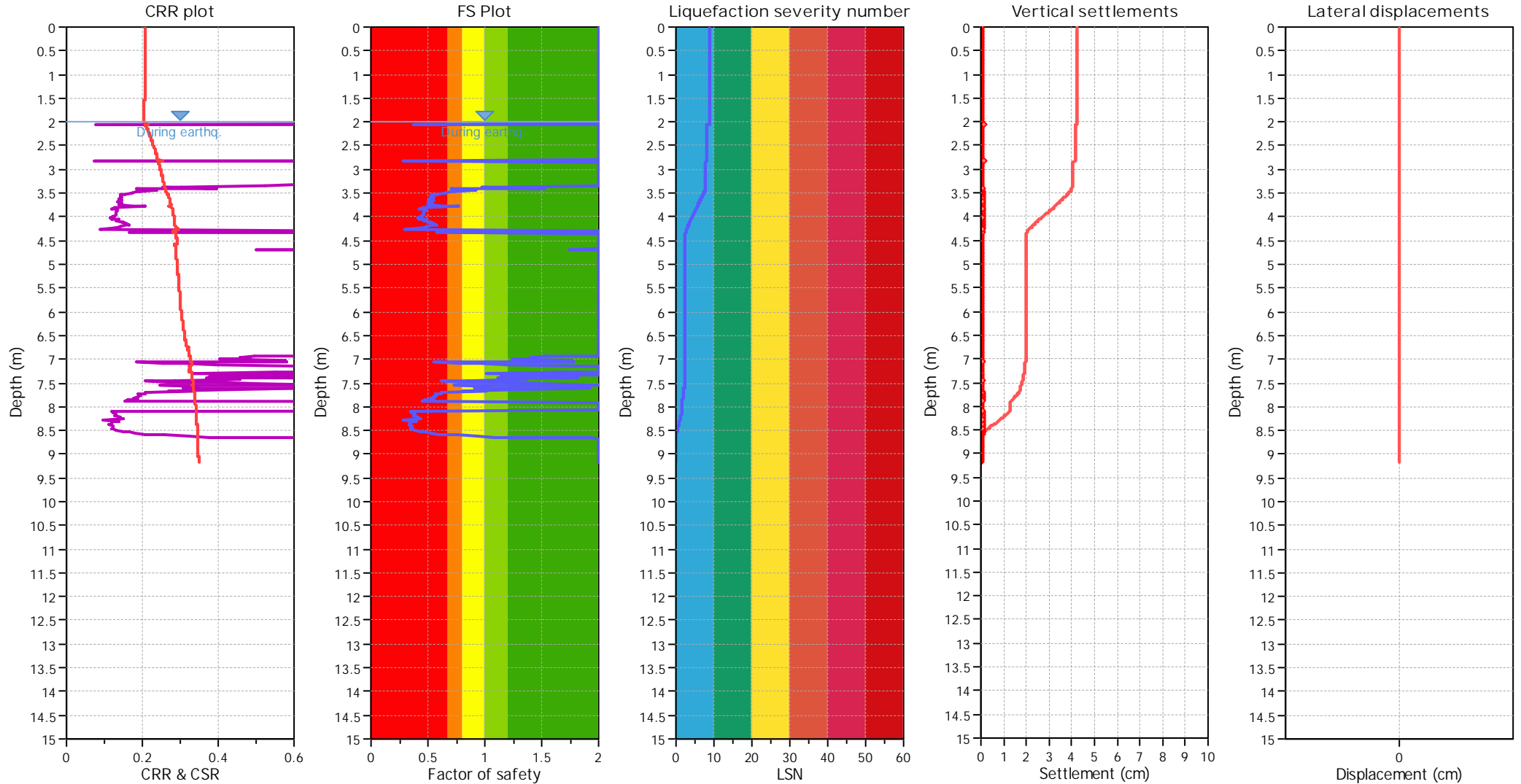
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on I _c value	I _c cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

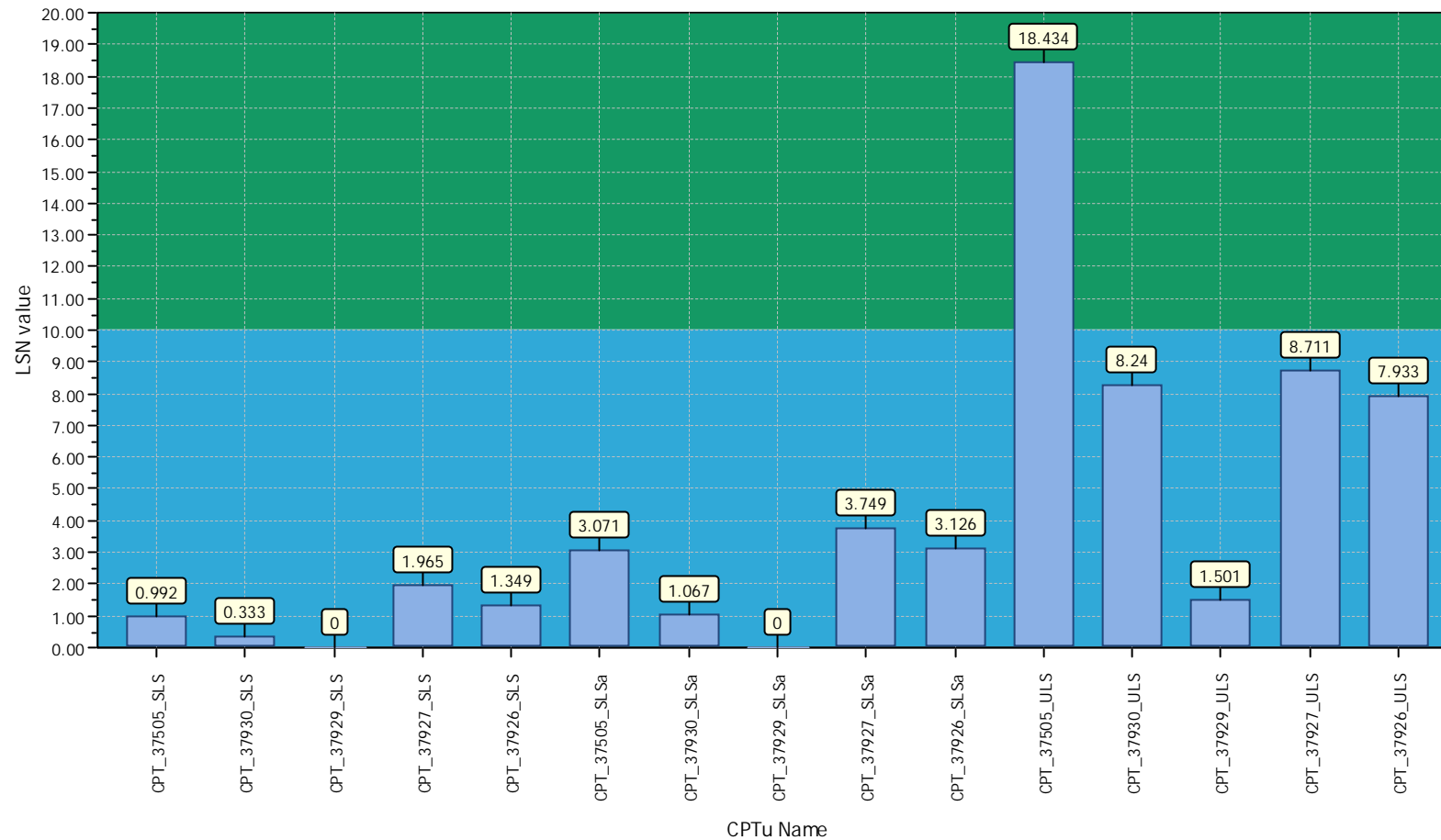
LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Project title : CLiq Analysis

Location : Nearby CPTs (NZGD) - 67 Mapleham Drive, Pegasus - West of Site

Overall Liquefaction Severity Number report



LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

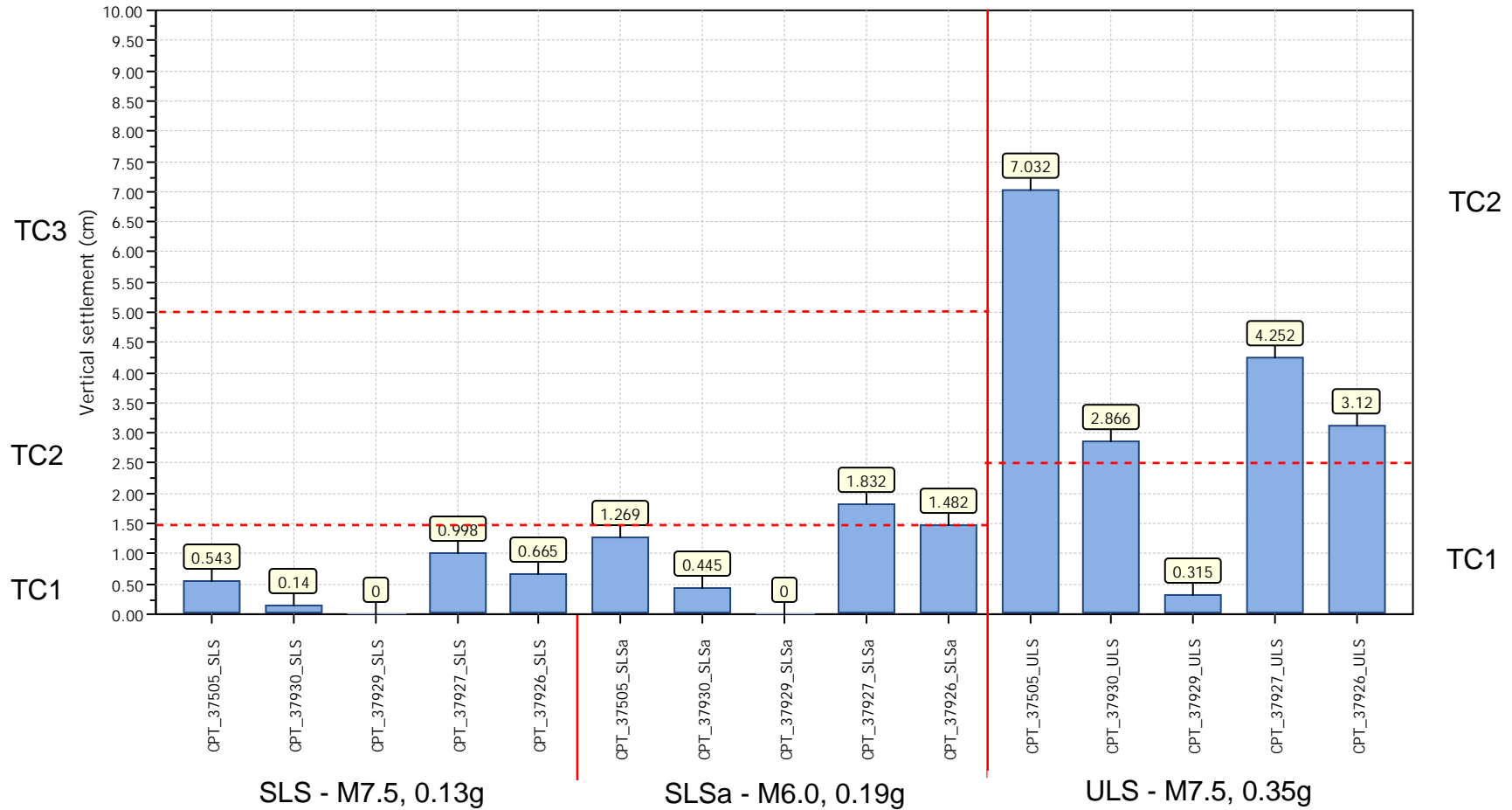
Basic statistics

- Total CPT number: 15
- 93% little liquefaction
- 7% minor liquefaction
- 0% moderate liquefaction
- 0% moderate to major liquefaction
- 0% major liquefaction
- 0% severe liquefaction

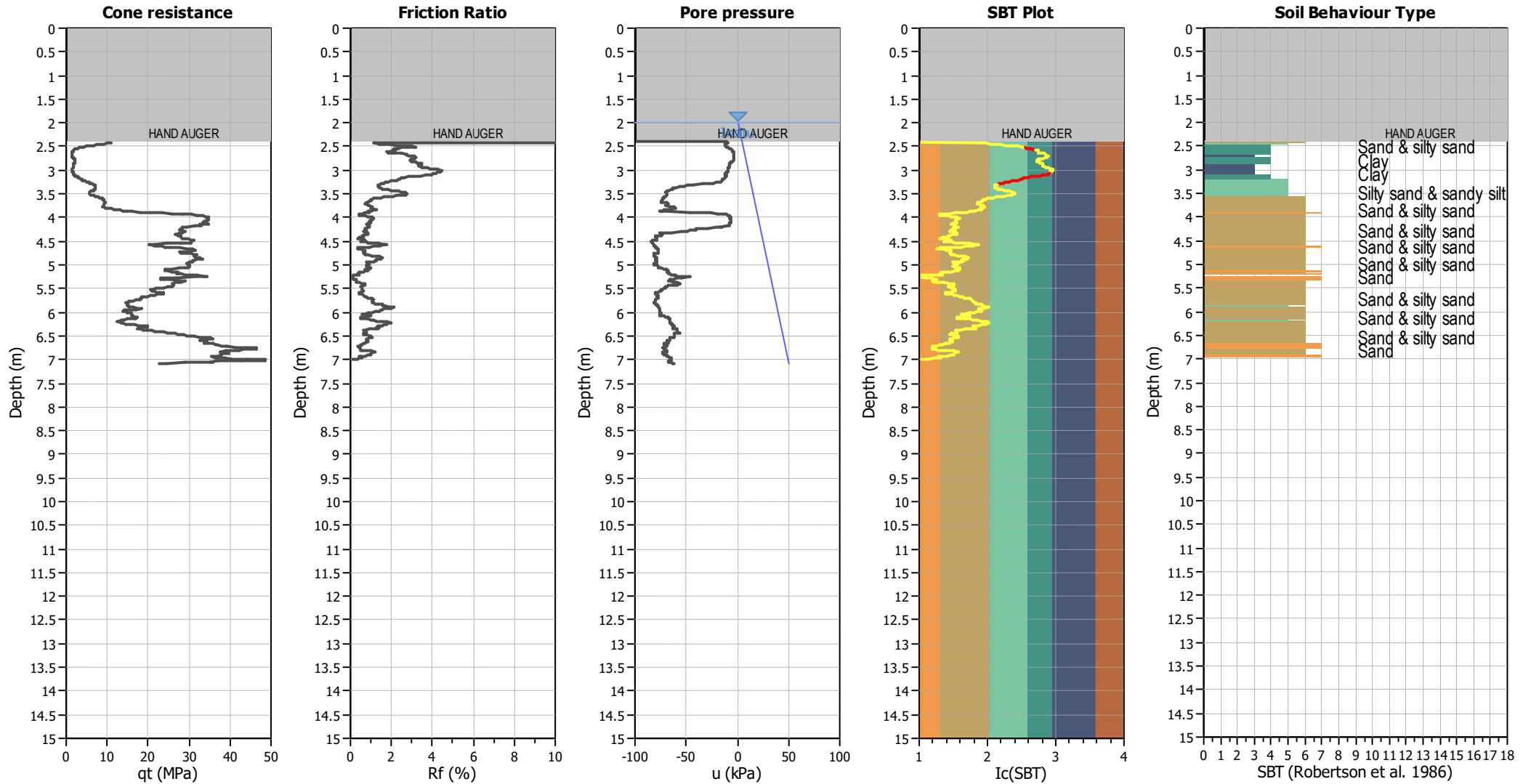
Project title : CLiq Analysis

Location : Nearby CPTs (NZGD) - 67 Mapleham Drive, Pegasus - West of Site

Overall vertical settlements report



CPT basic interpretation plots



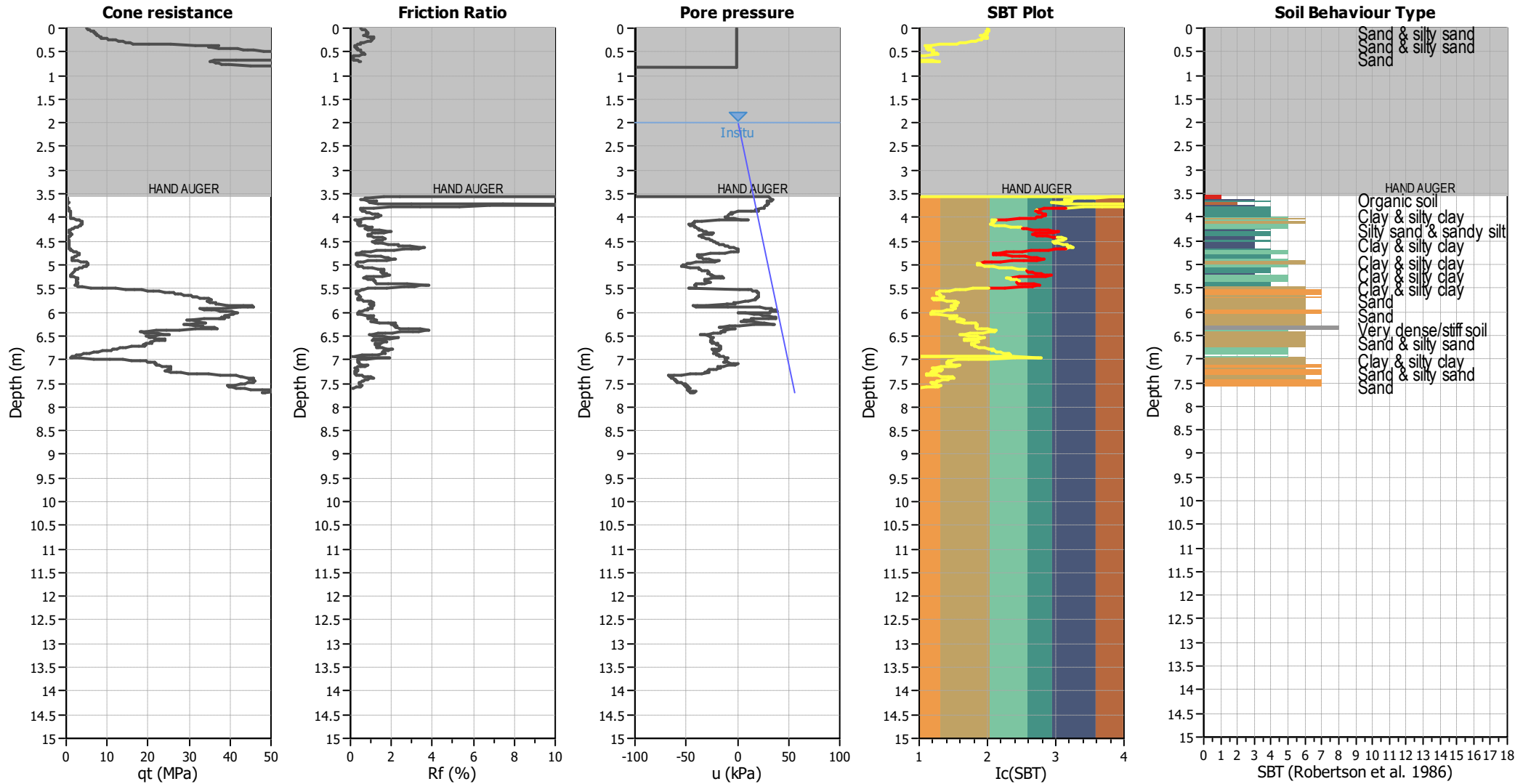
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots



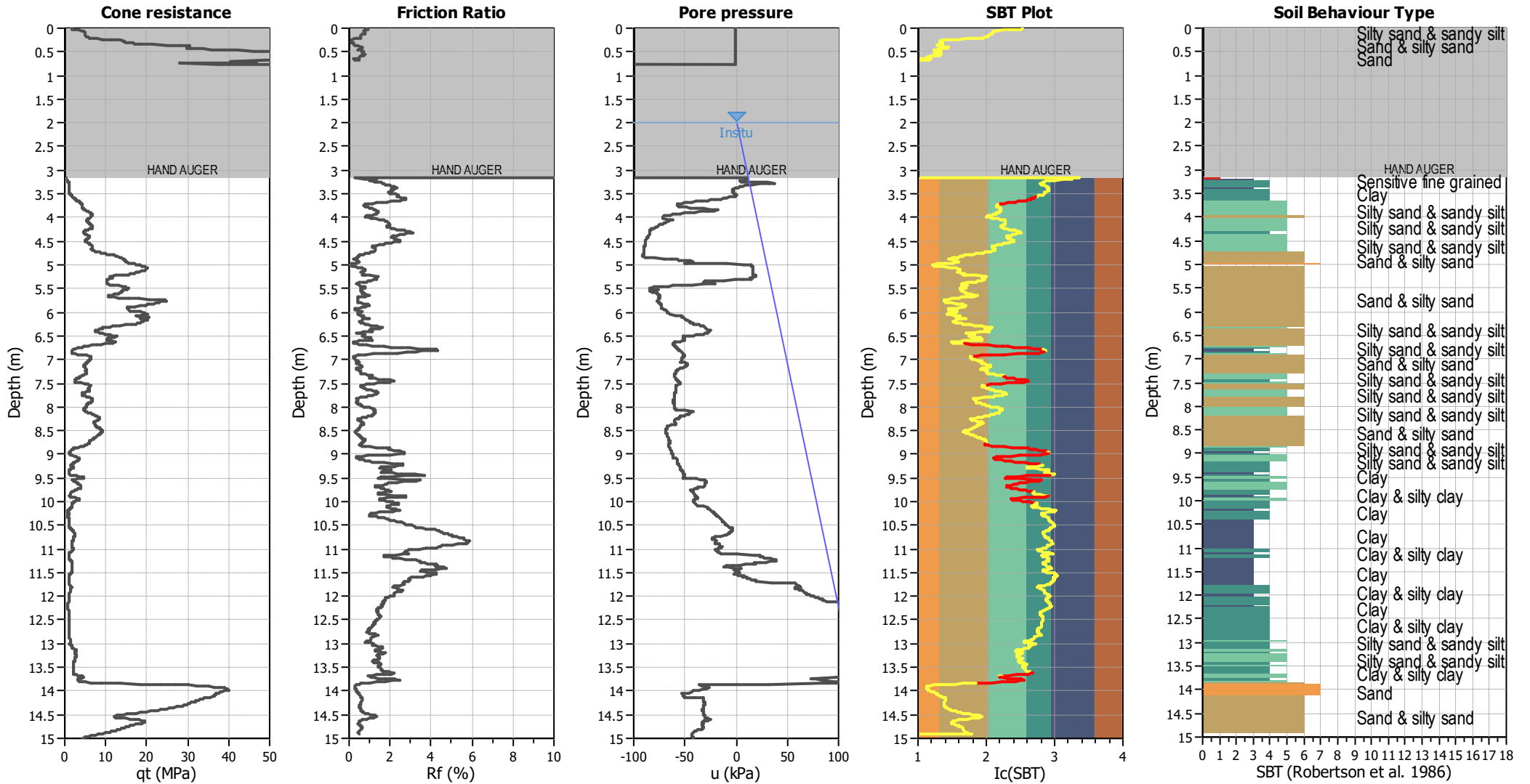
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots



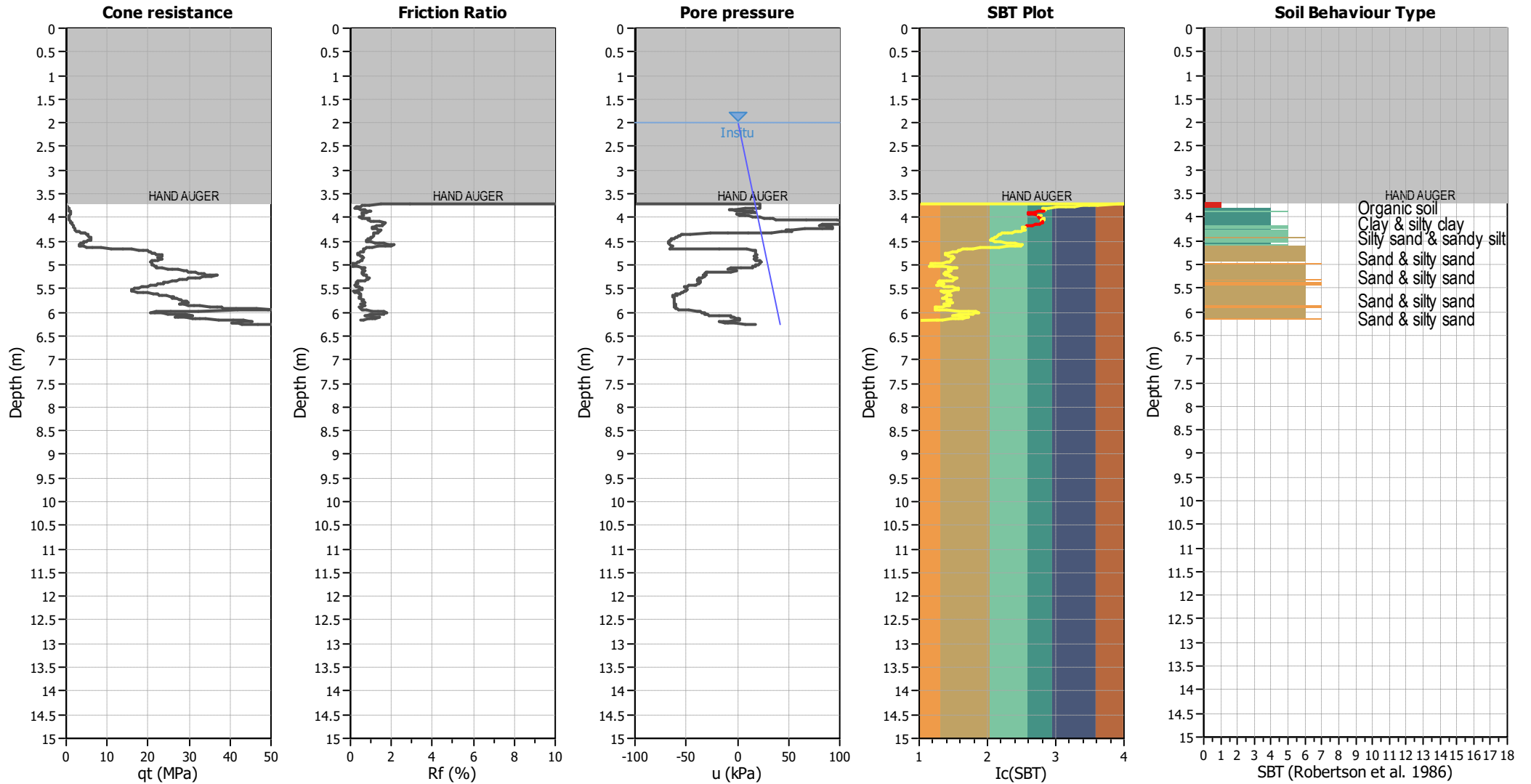
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _q applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

■ 1. Sensitive fine grained	■ 4. Clayey silt to silty	■ 7. Gravely sand to sand
■ 2. Organic material	■ 5. Silty sand to sandy silt	■ 8. Very stiff sand to
■ 3. Clay to silty clay	■ 6. Clean sand to silty sand	■ 9. Very stiff fine grained

CPT basic interpretation plots



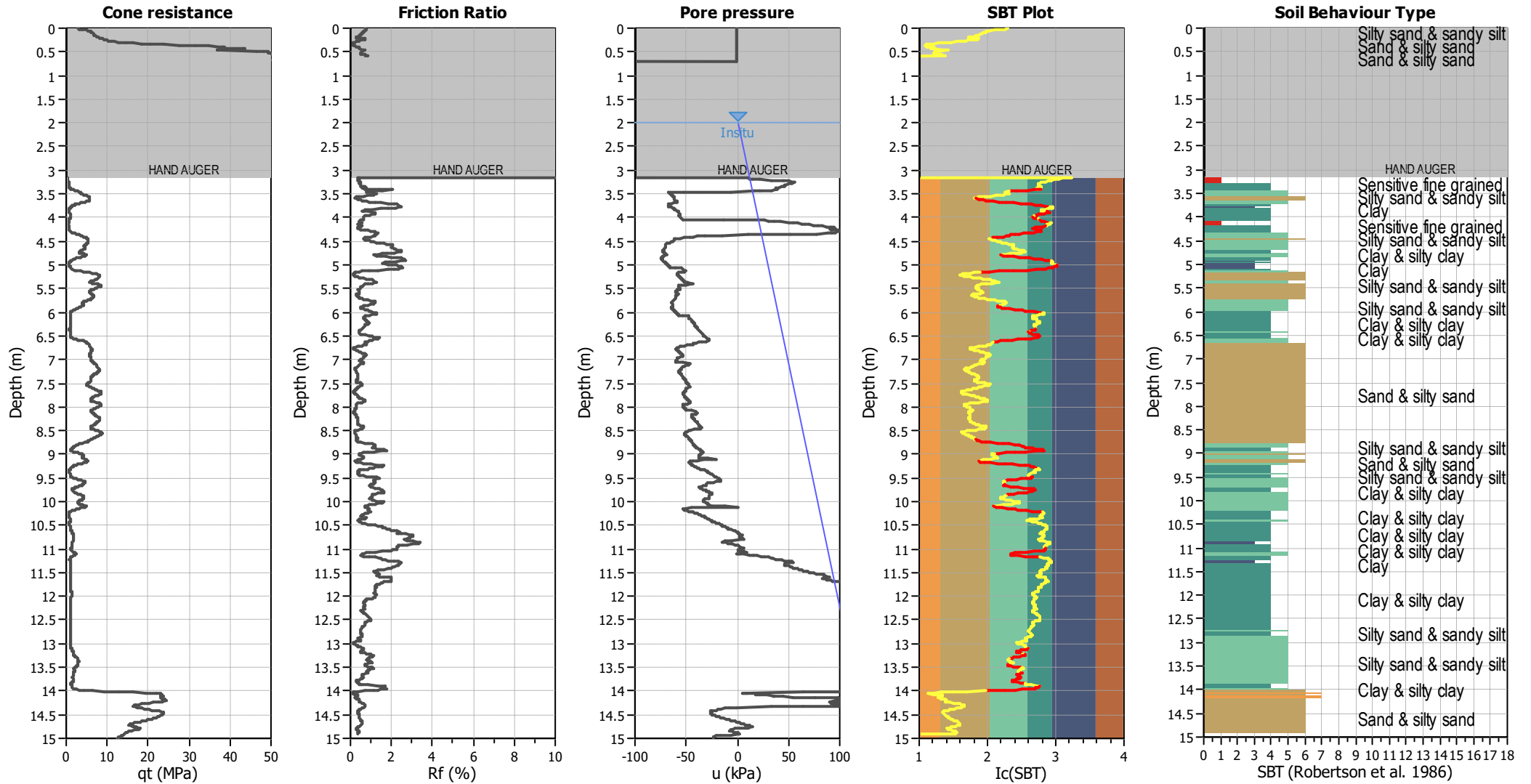
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots



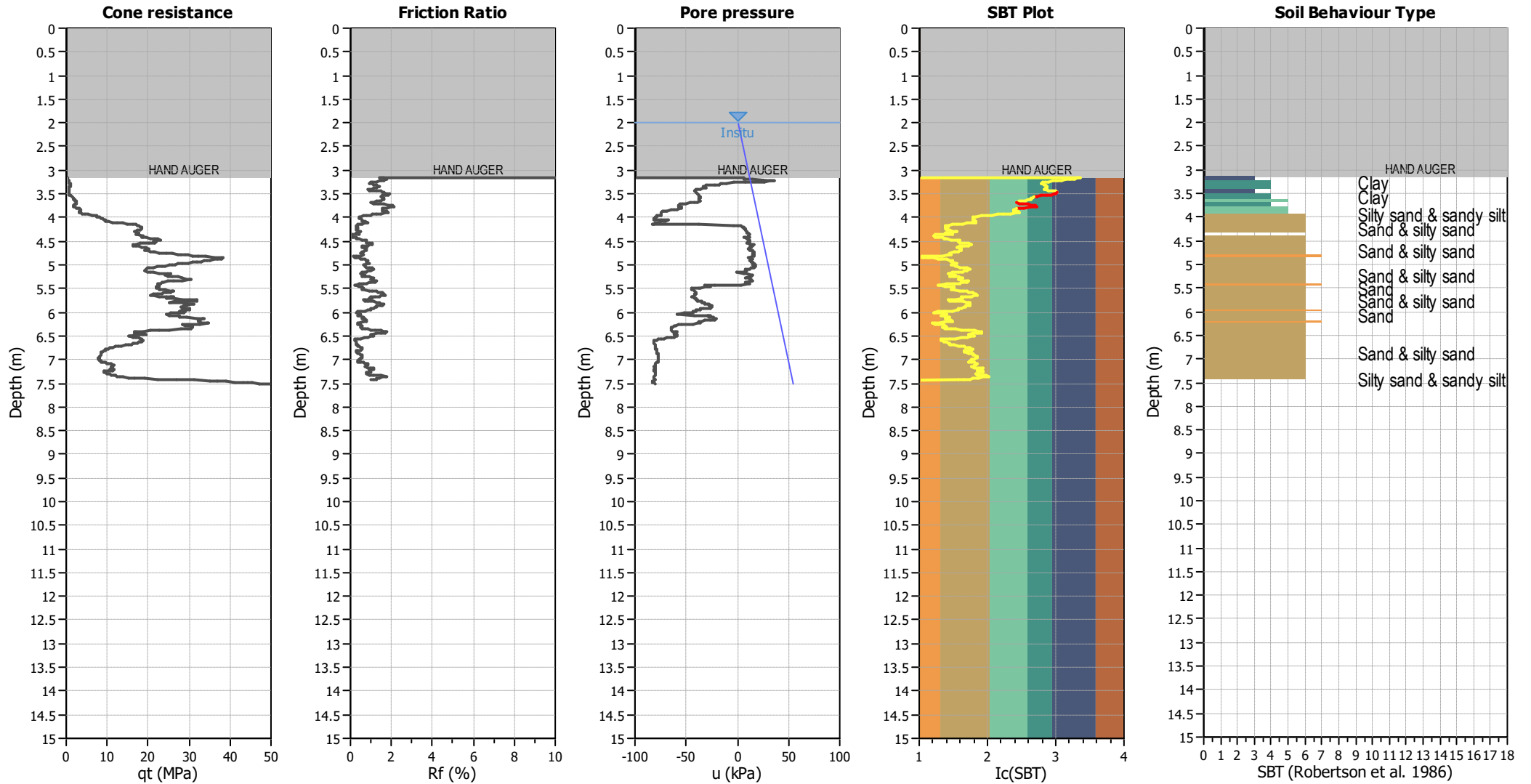
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _q applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots



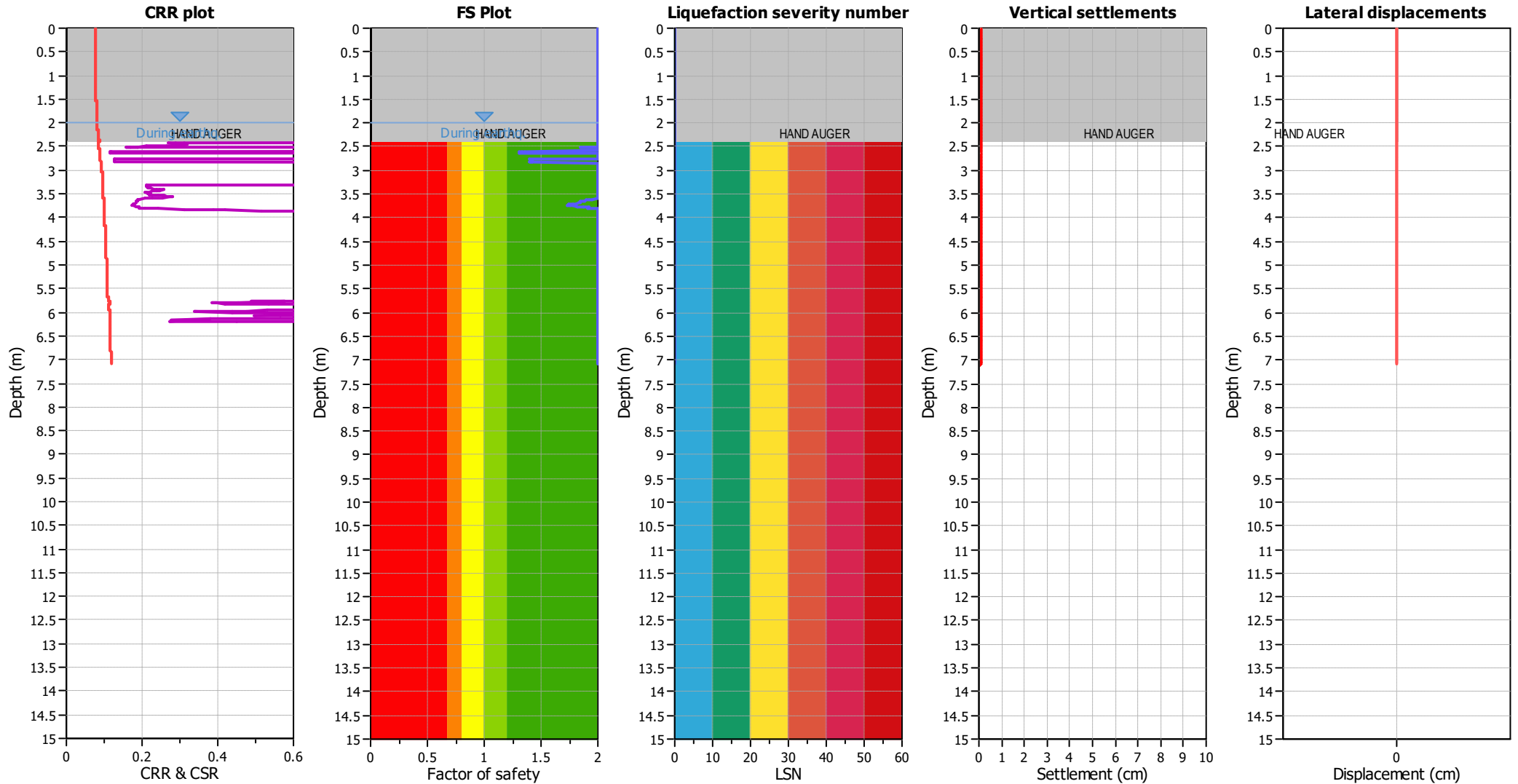
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

SBT legend

■ 1. Sensitive fine grained	■ 4. Clayey silt to silty	■ 7. Gravely sand to sand
■ 2. Organic material	■ 5. Silty sand to sandy silt	■ 8. Very stiff sand to
■ 3. Clay to silty clay	■ 6. Clean sand to silty sand	■ 9. Very stiff fine grained

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

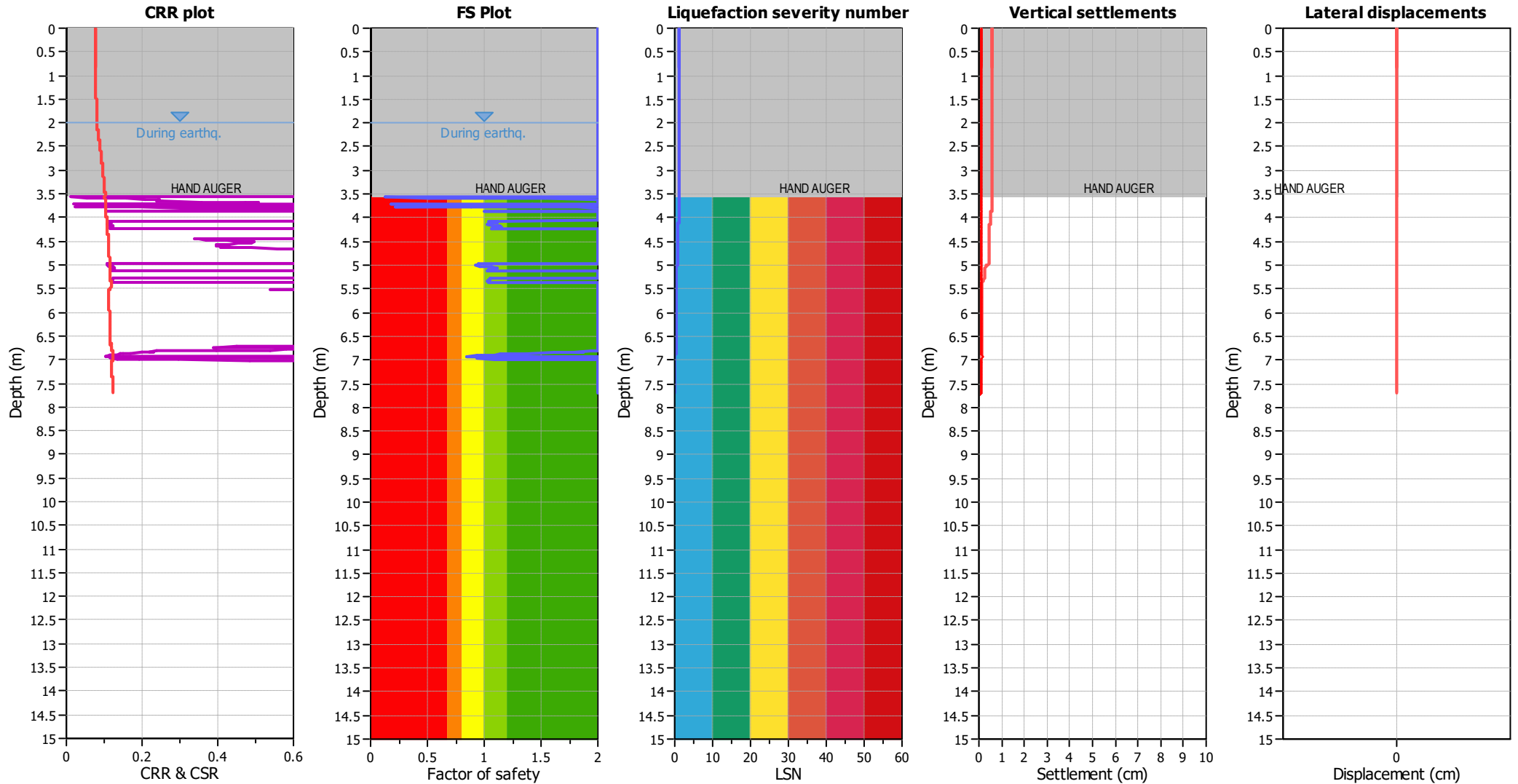
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_d applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

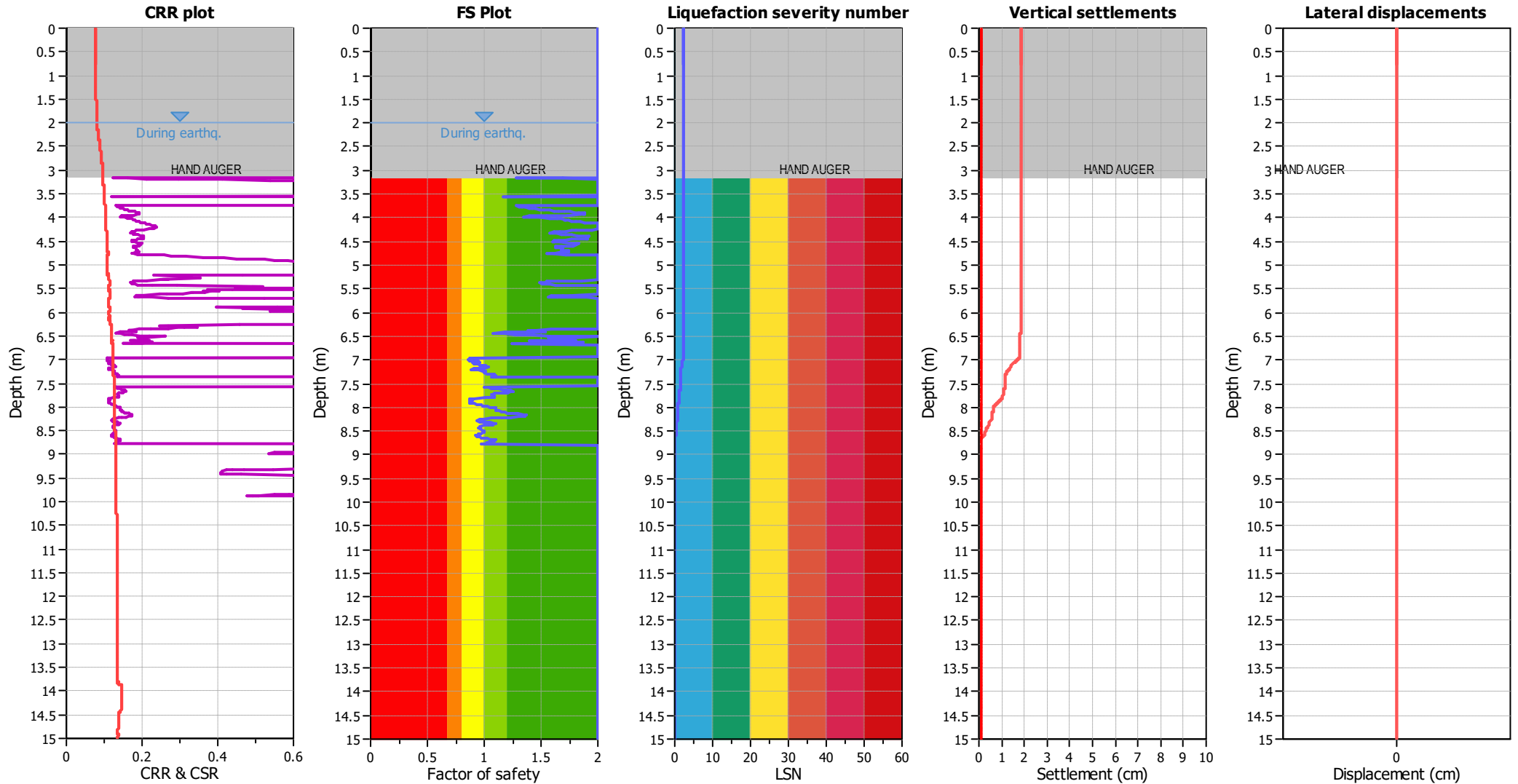
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

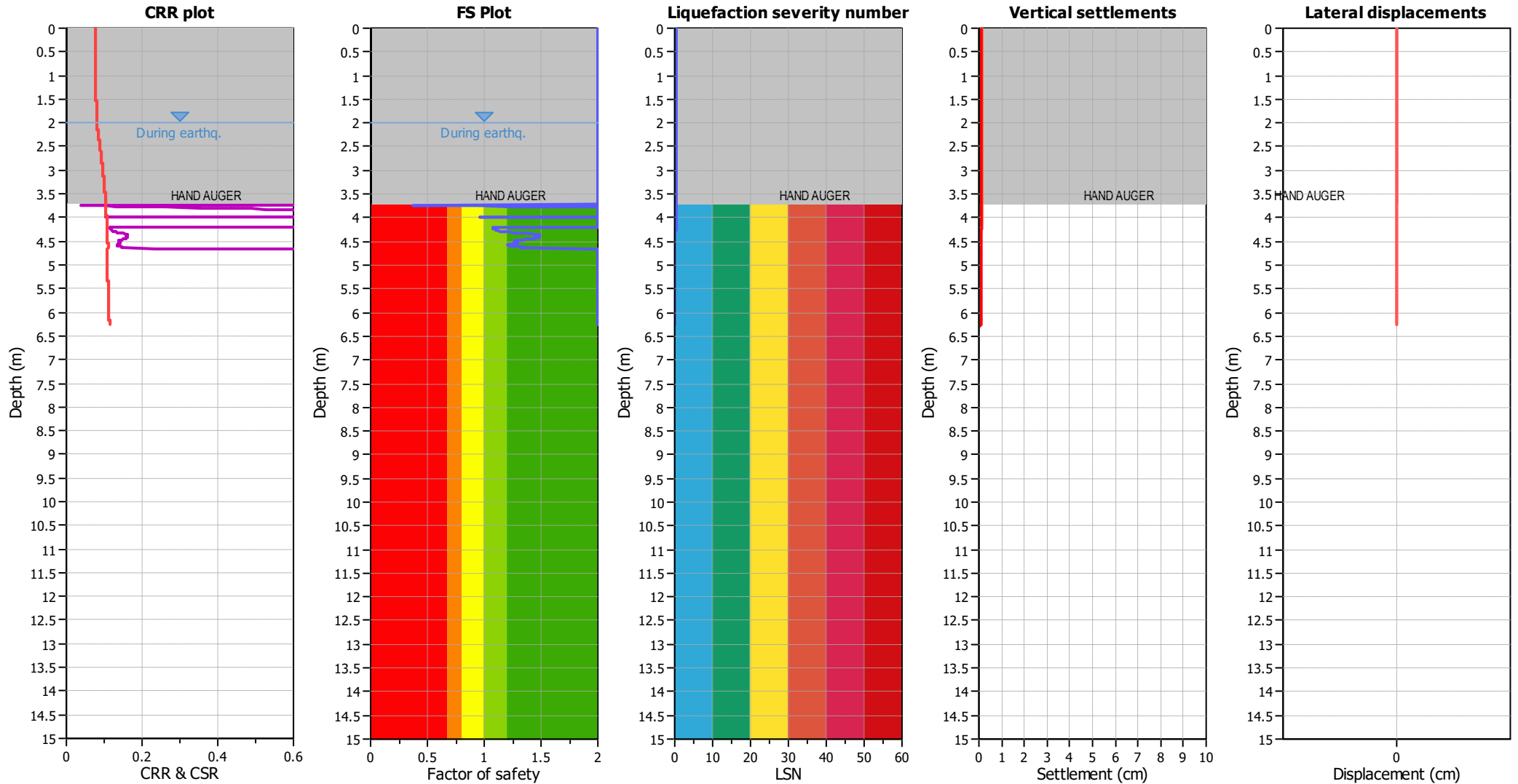
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

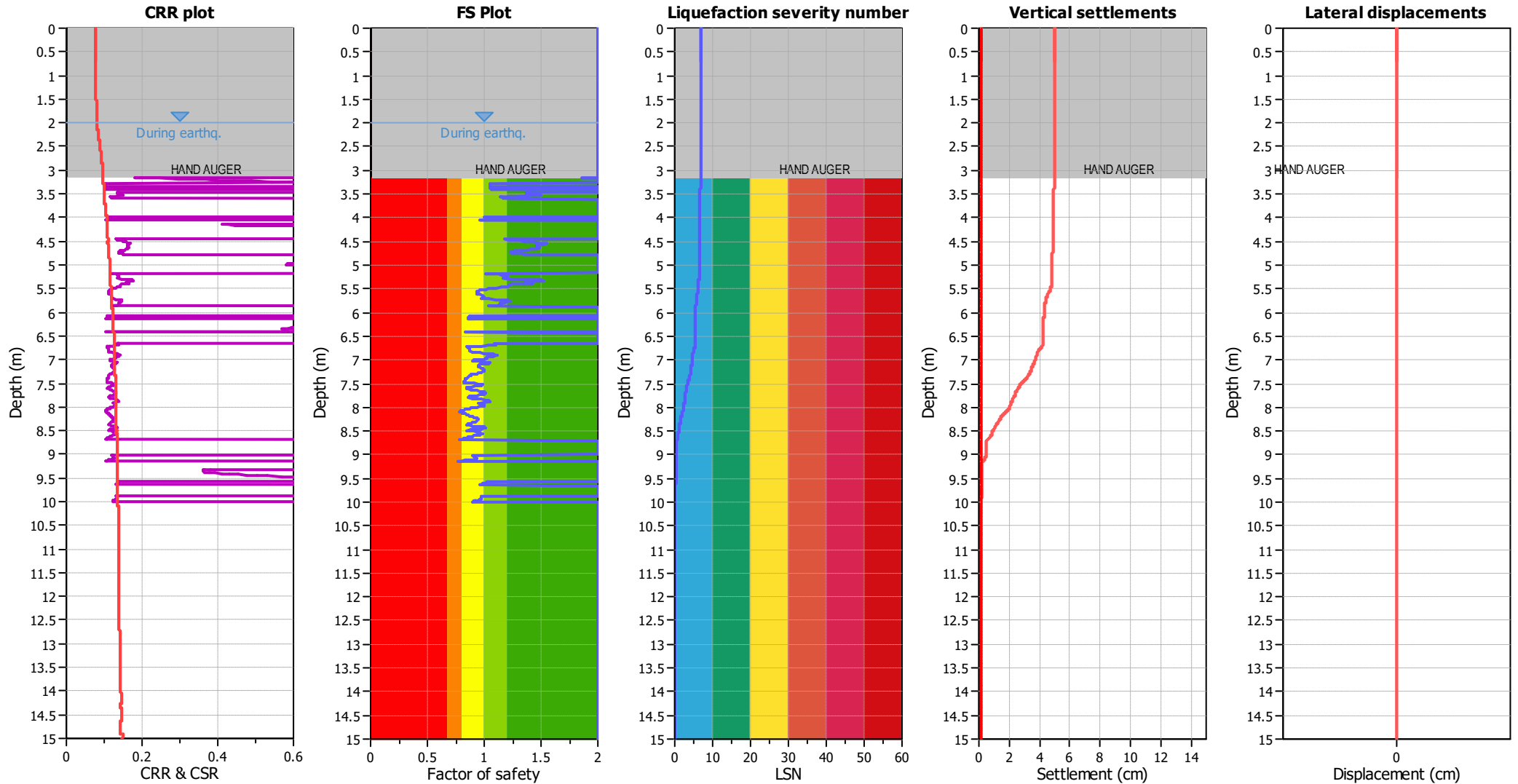
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

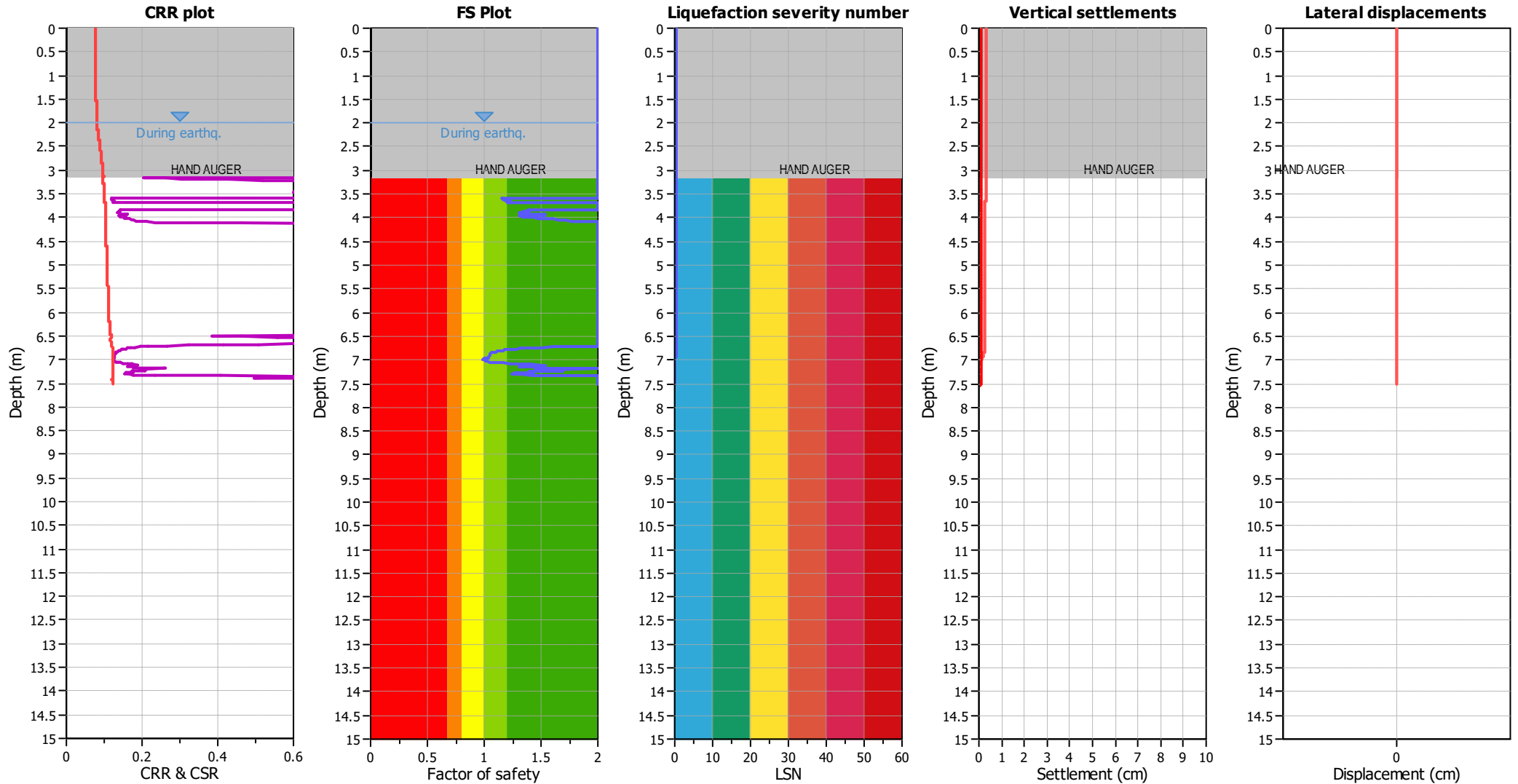
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.13	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

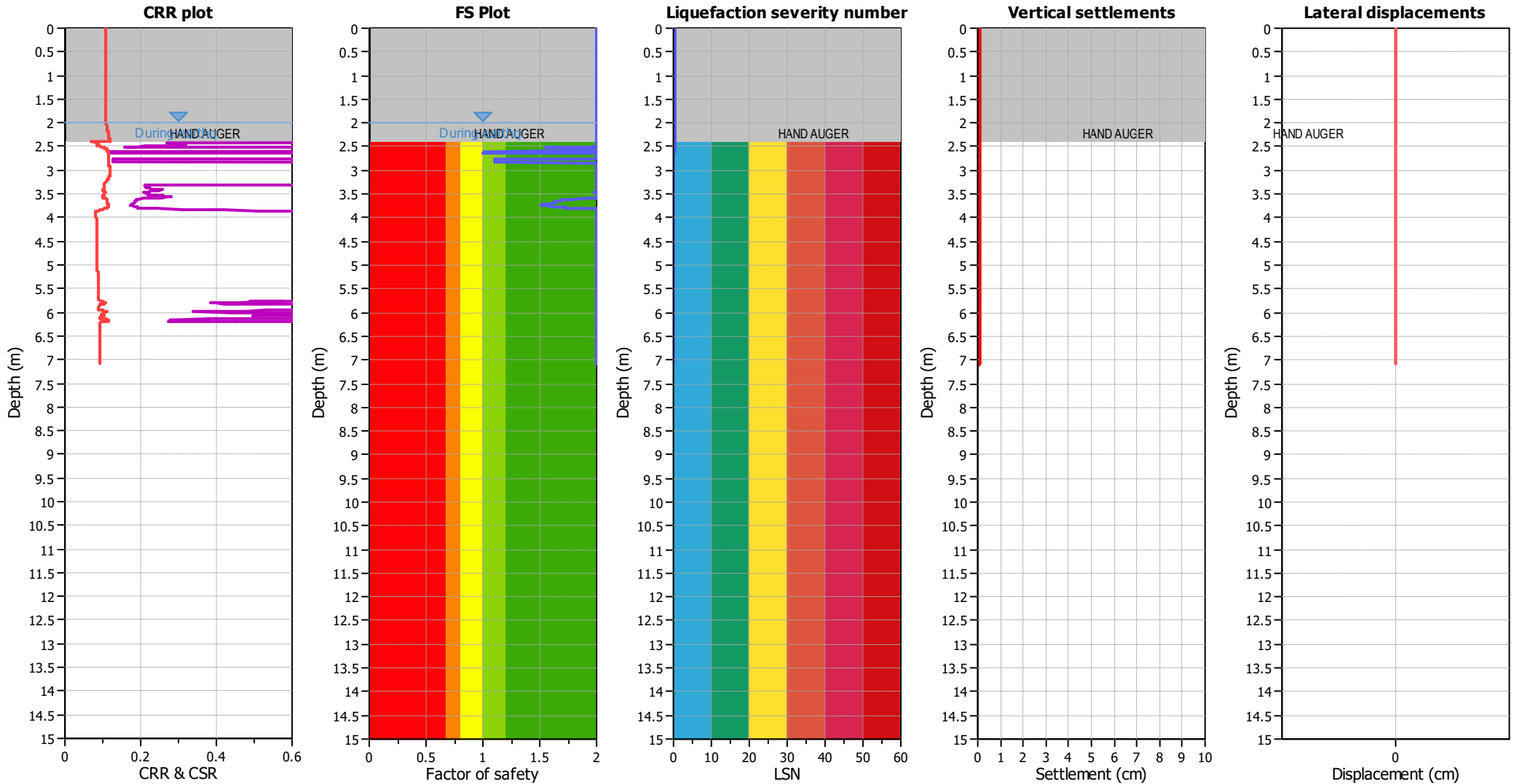
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

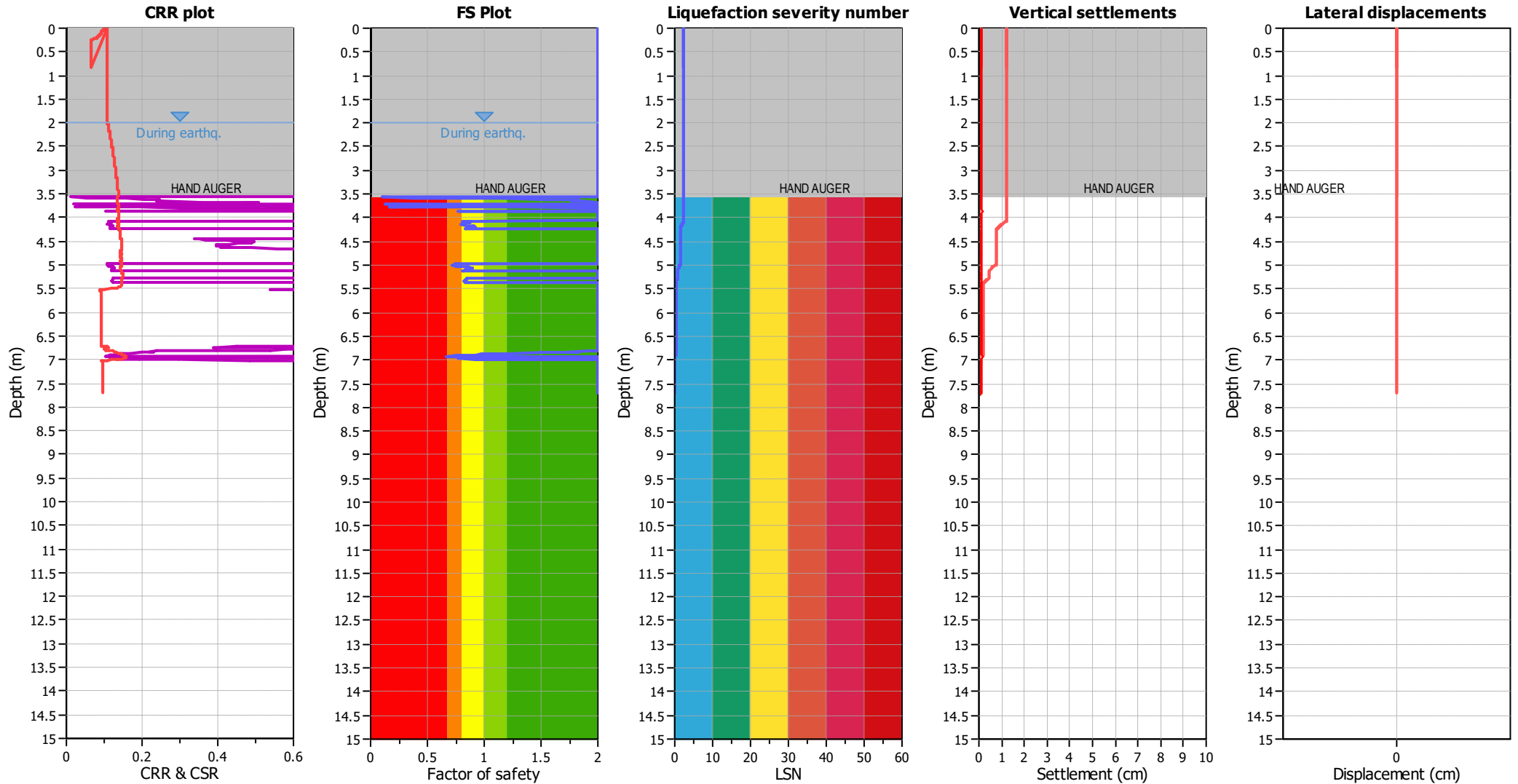
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

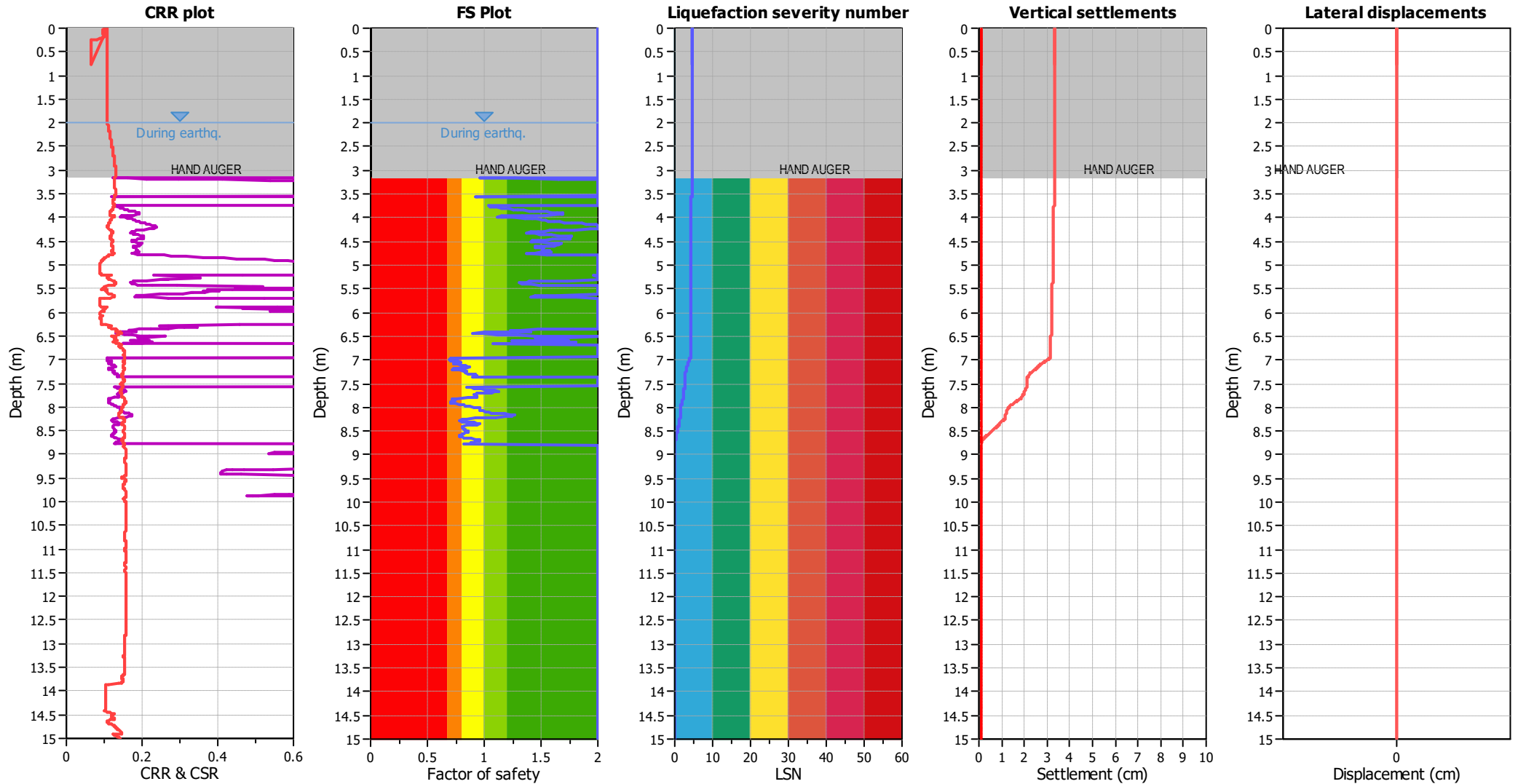
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

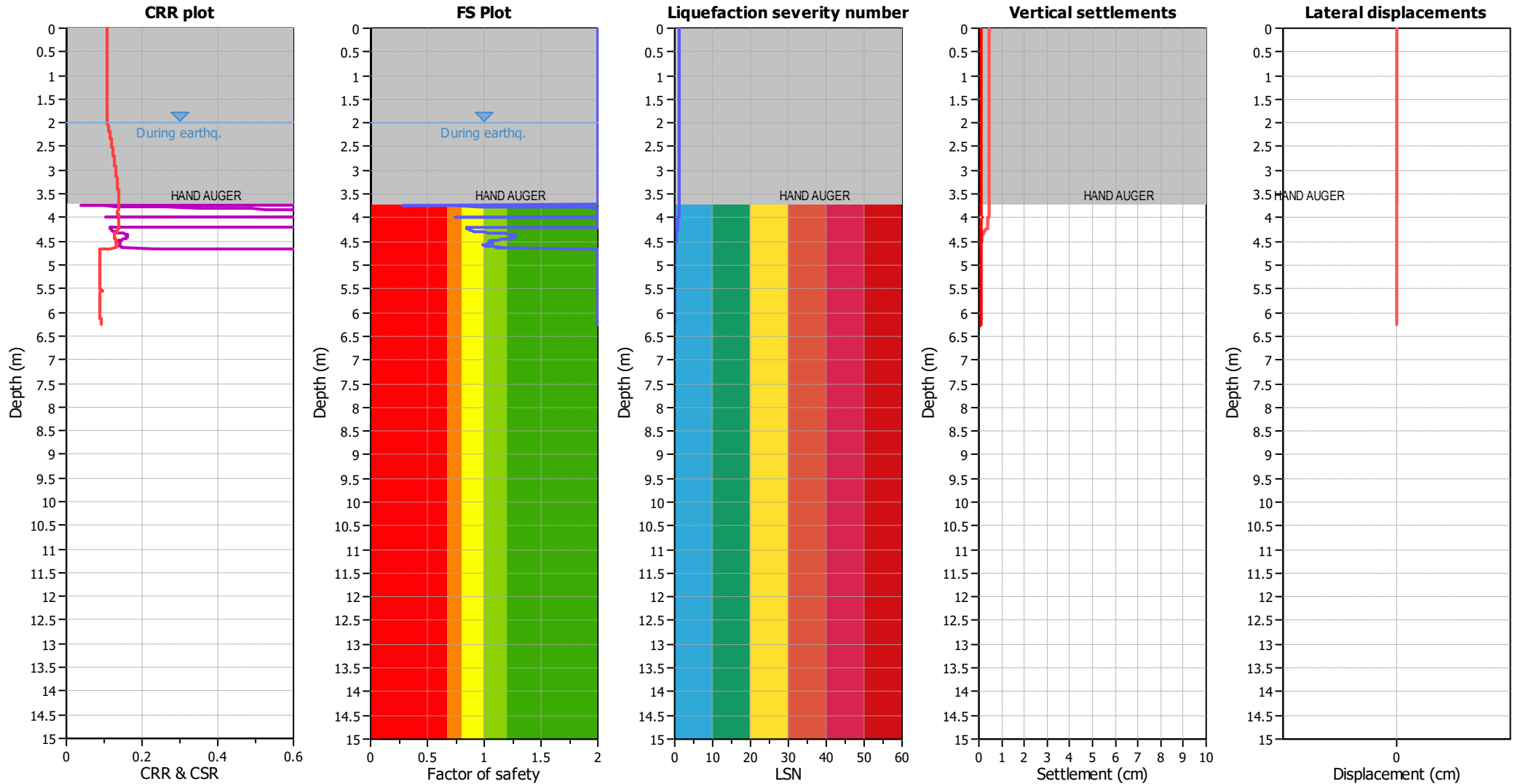
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

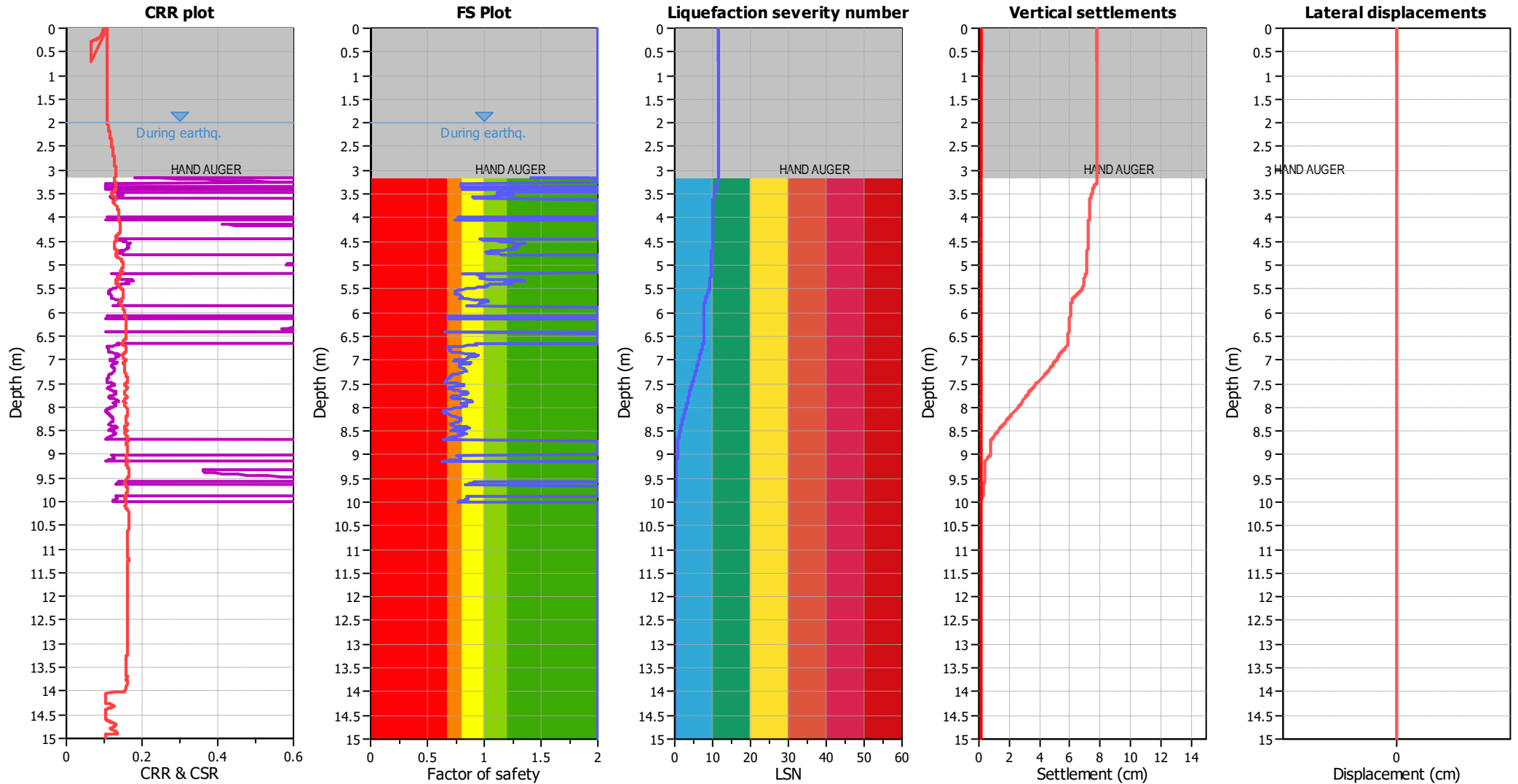
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

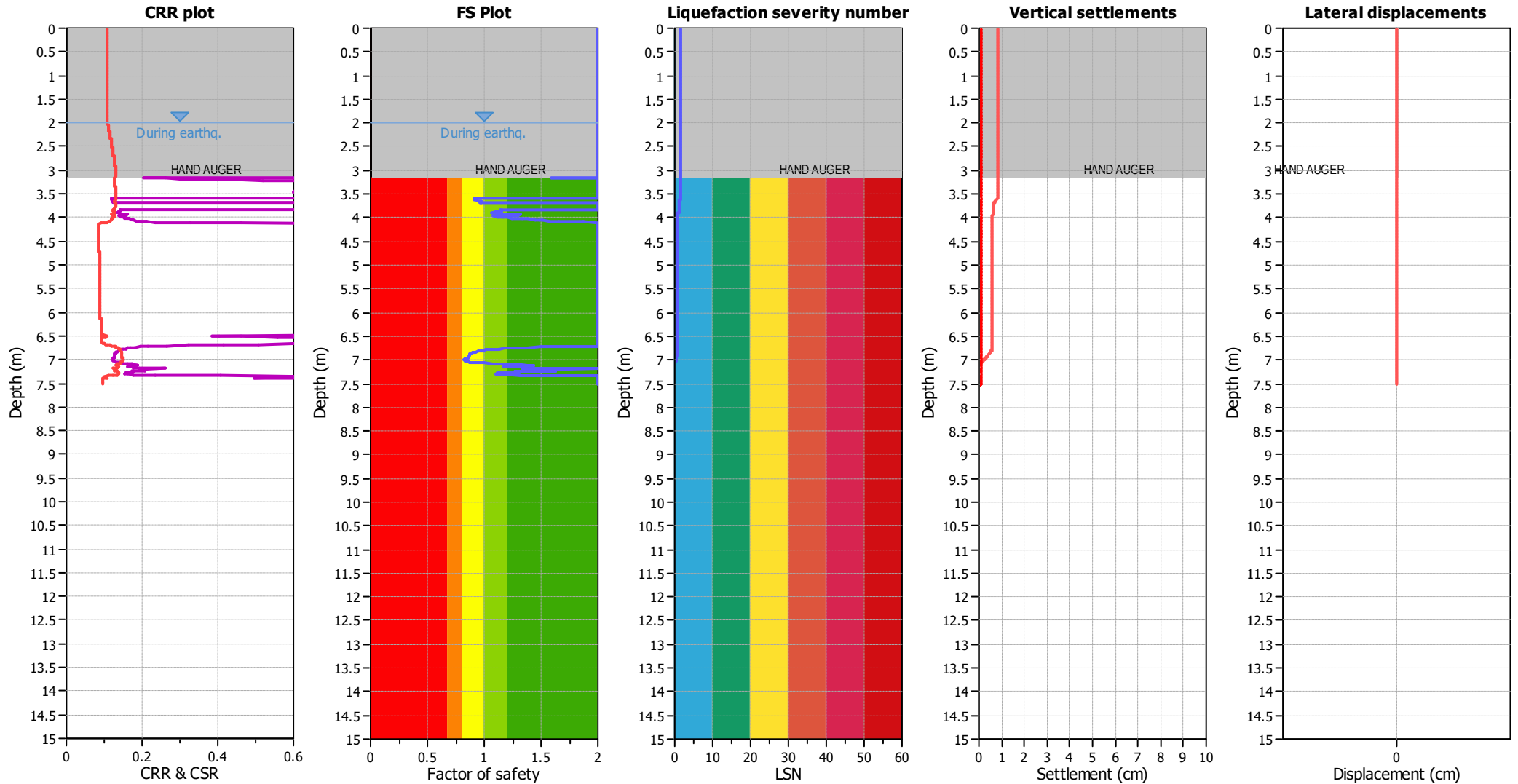
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	6.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.19	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

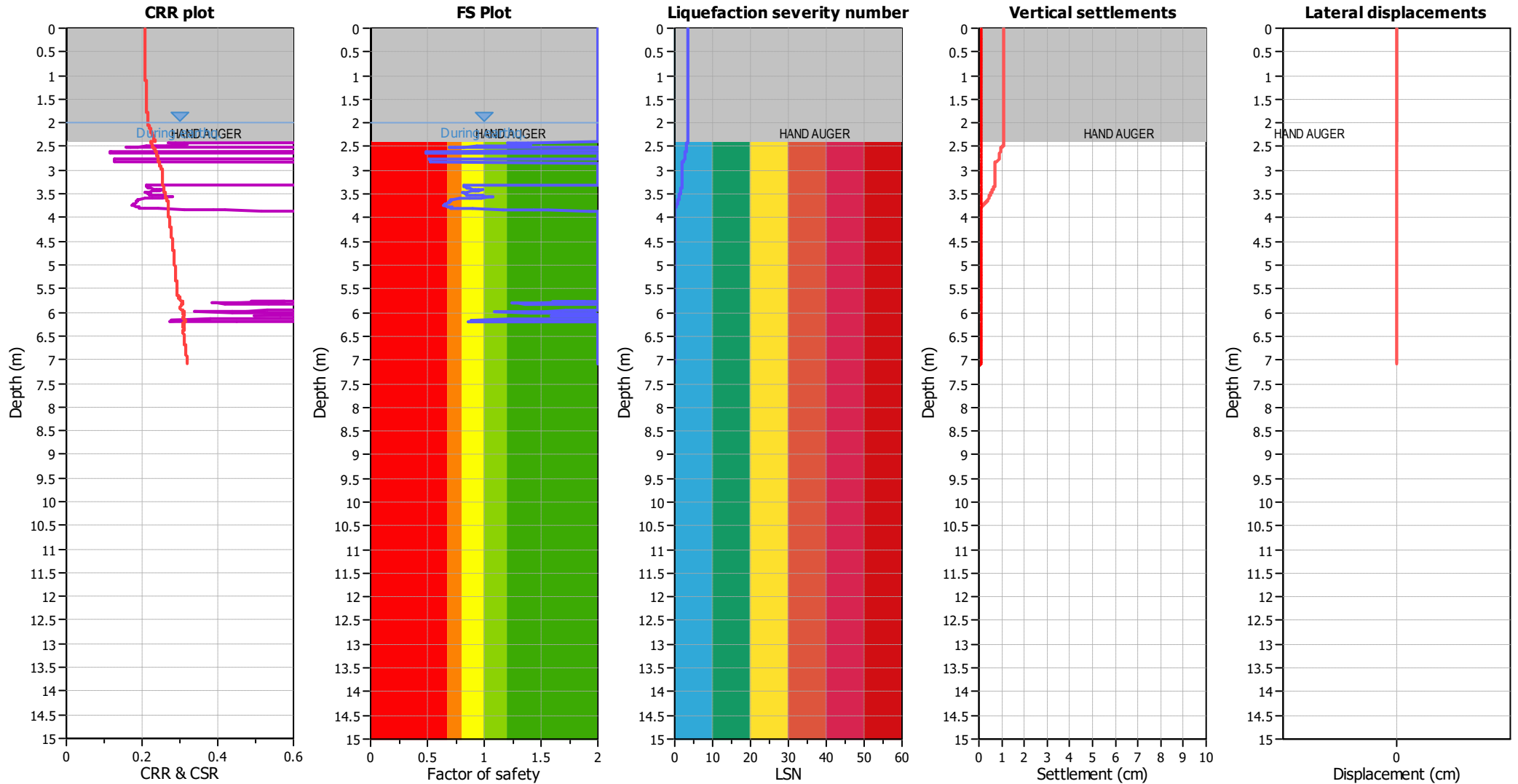
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

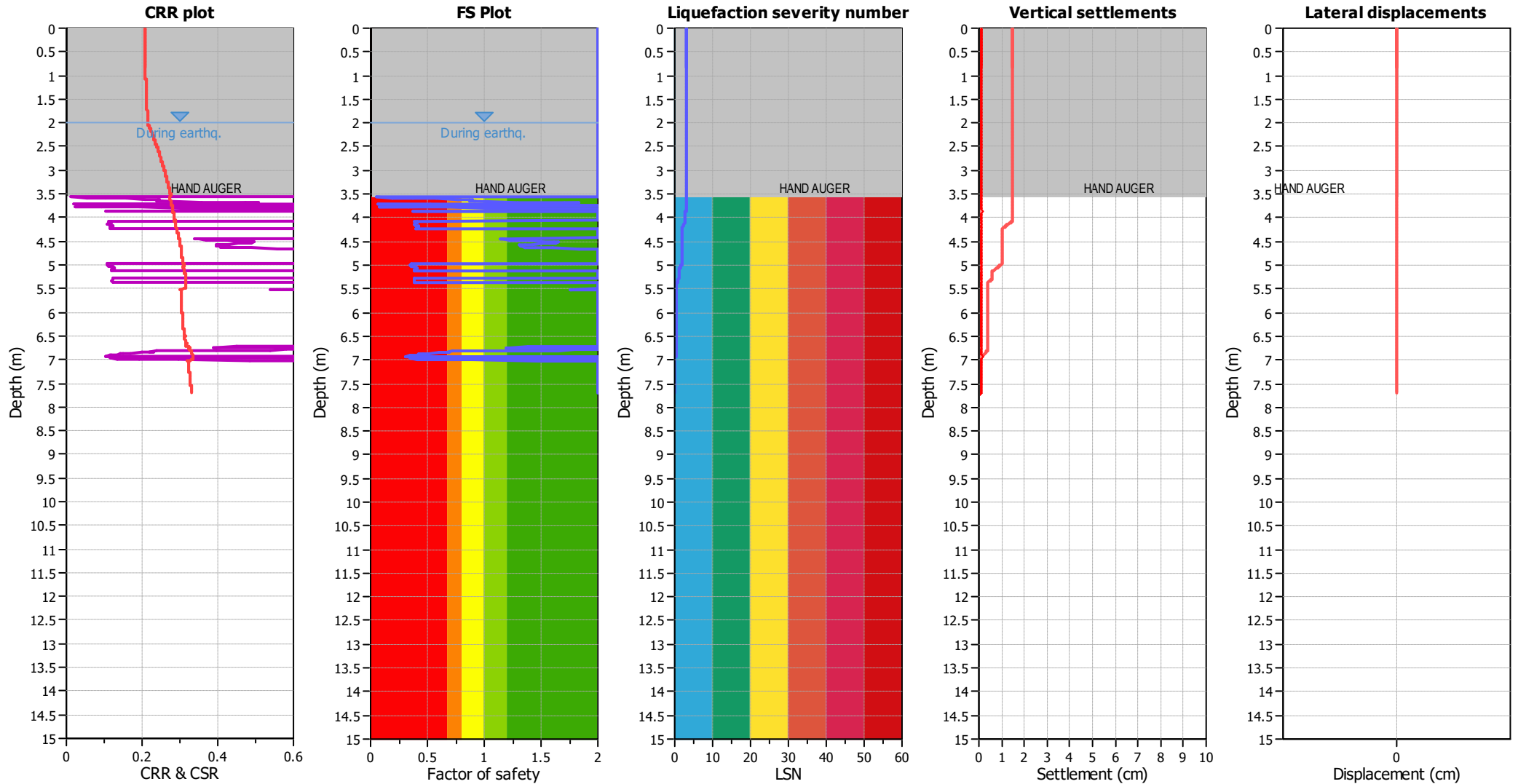
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

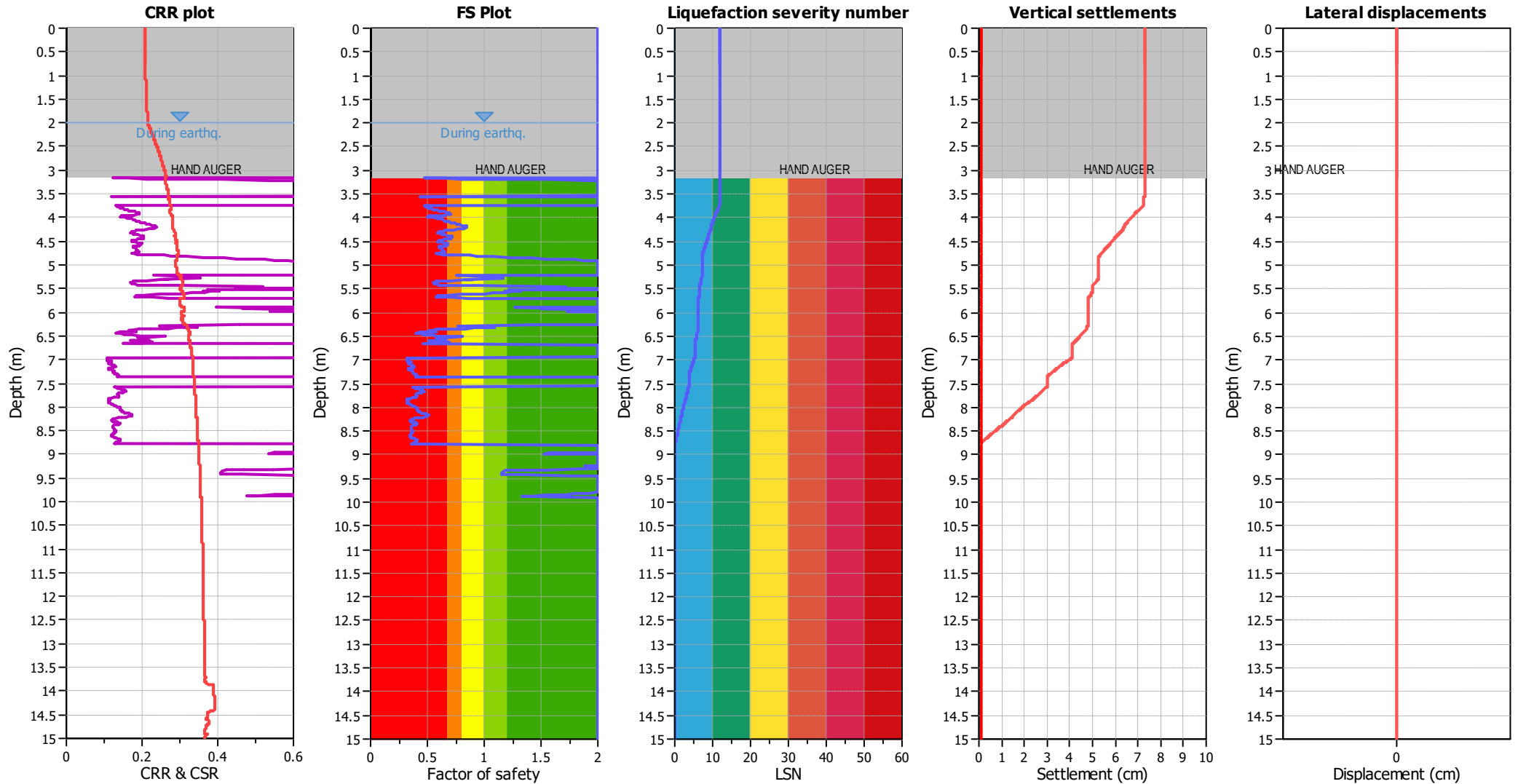
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

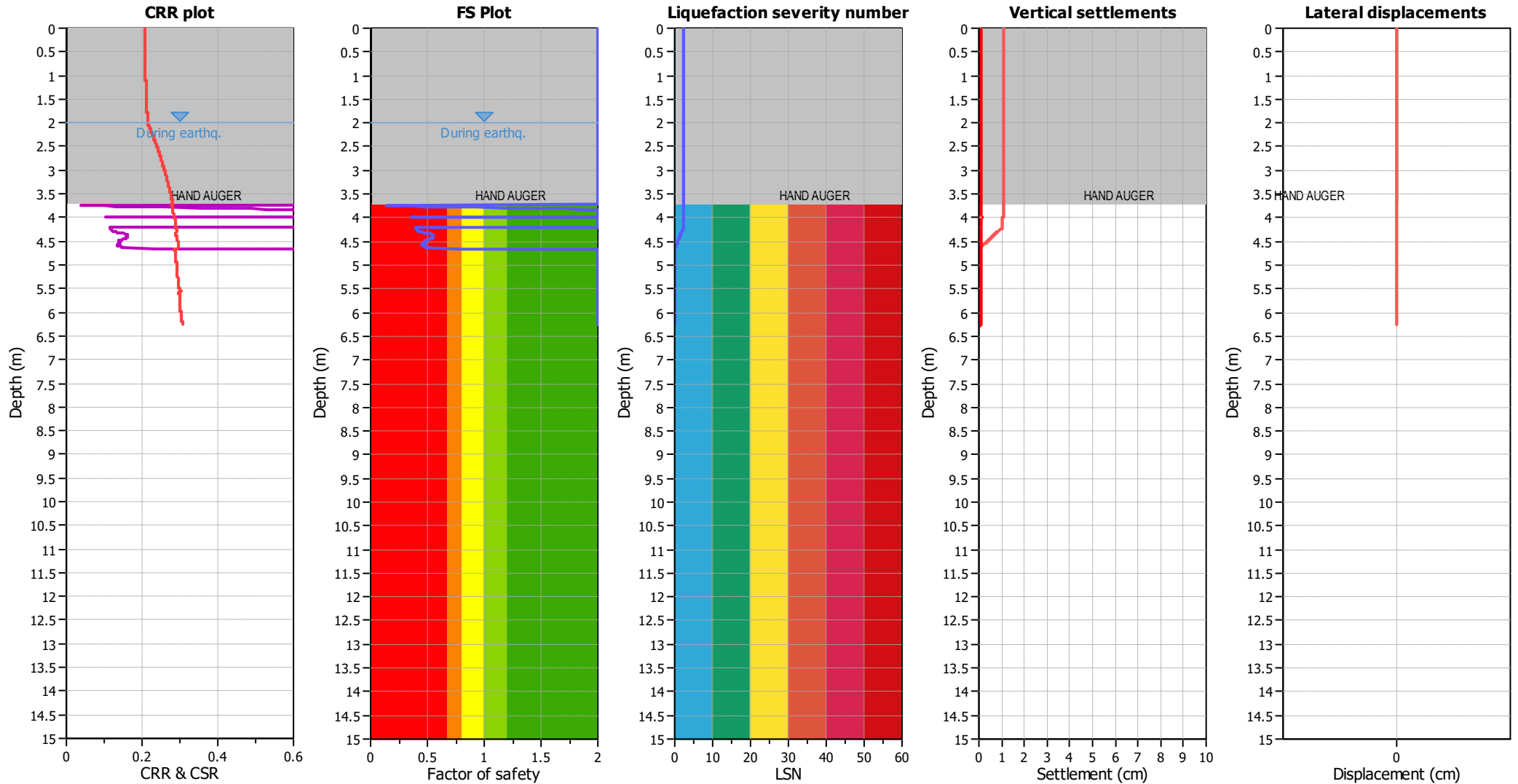
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

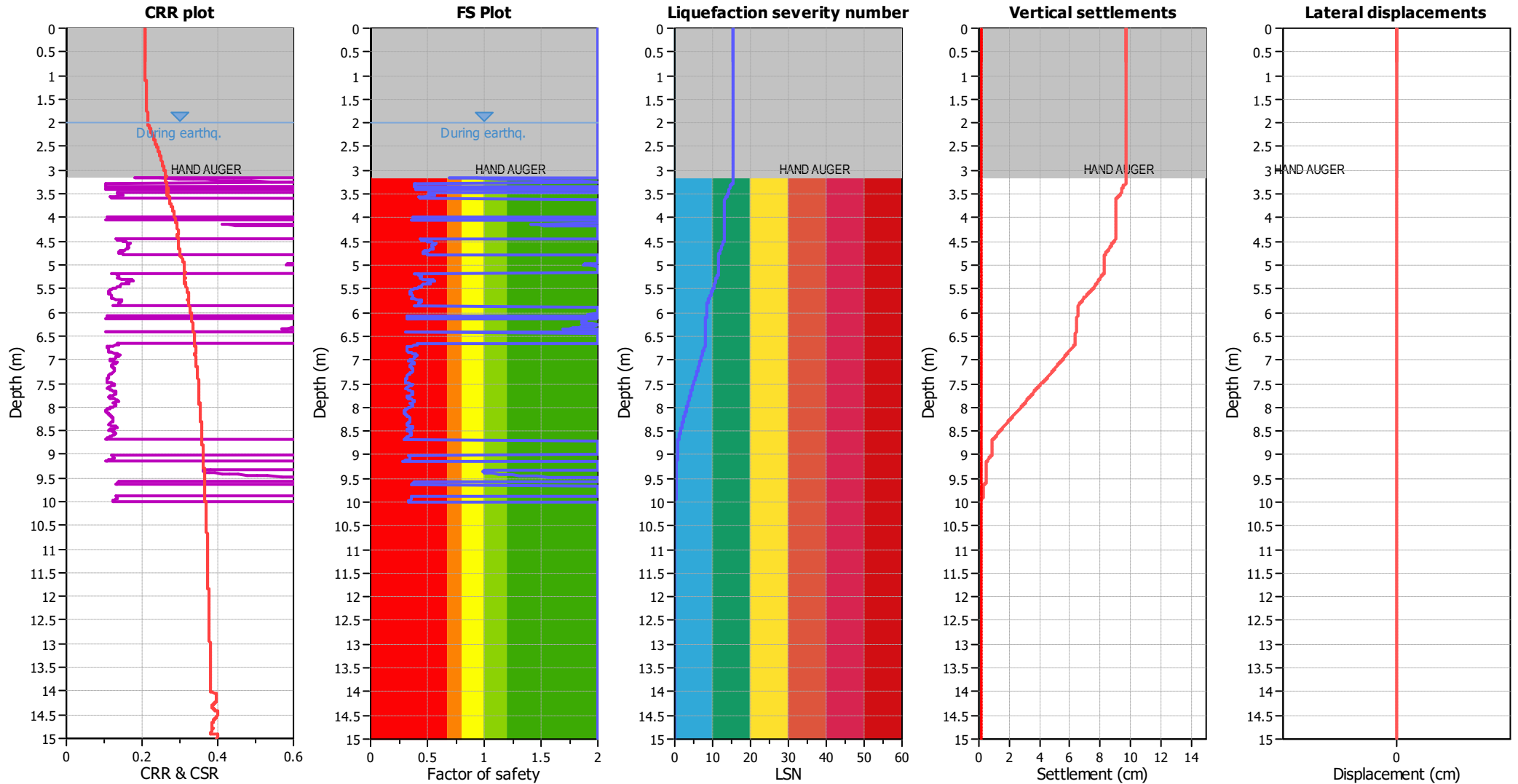
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_{σ} applied:	Yes
Earthquake magnitude M_w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

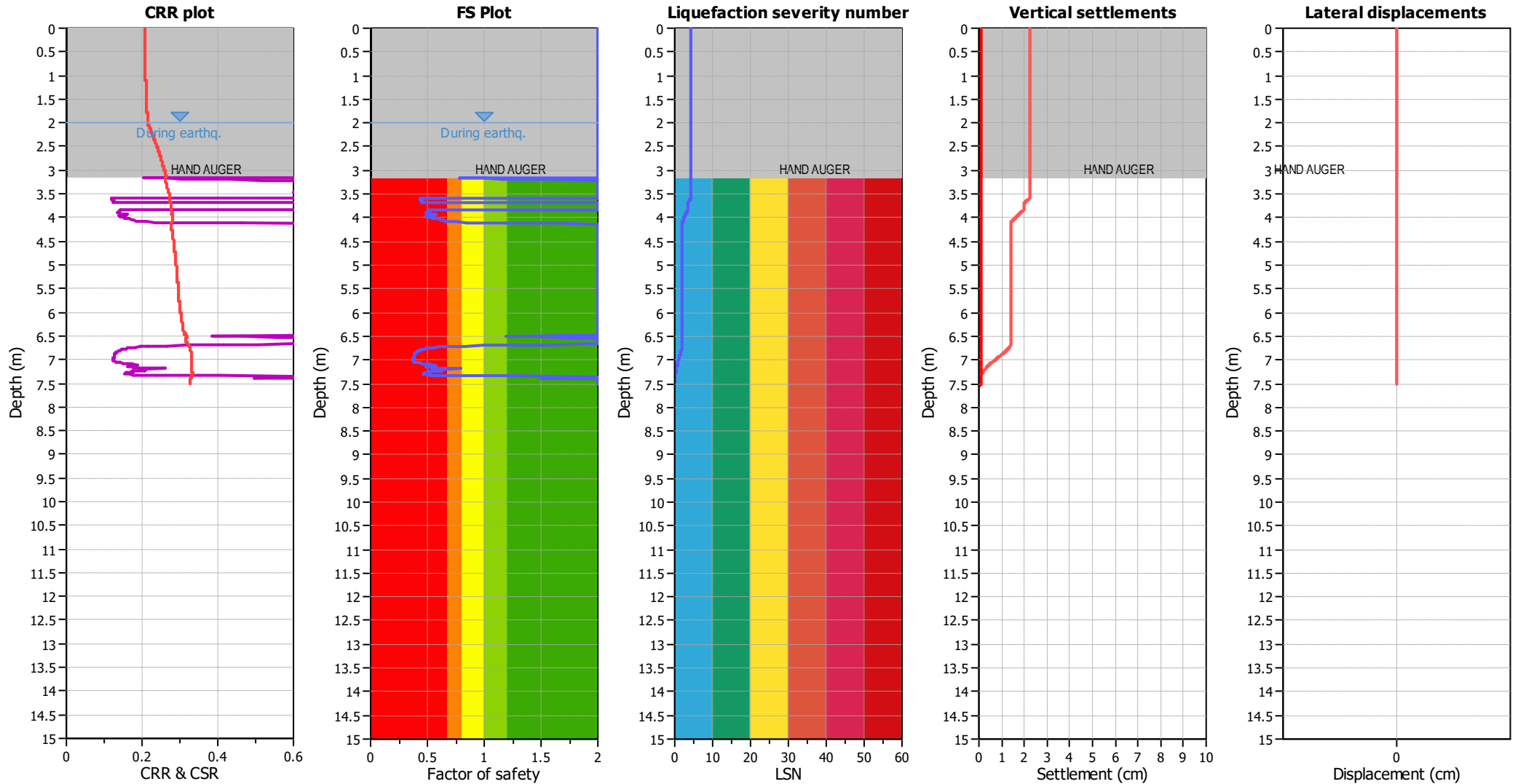
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	2.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	7.50	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sand & Clay
Peak ground acceleration:	0.35	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.00 m	Fill height:	N/A	Limit depth:	10.00 m

F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlike to liquefy
- Almost certain it will not liquefy

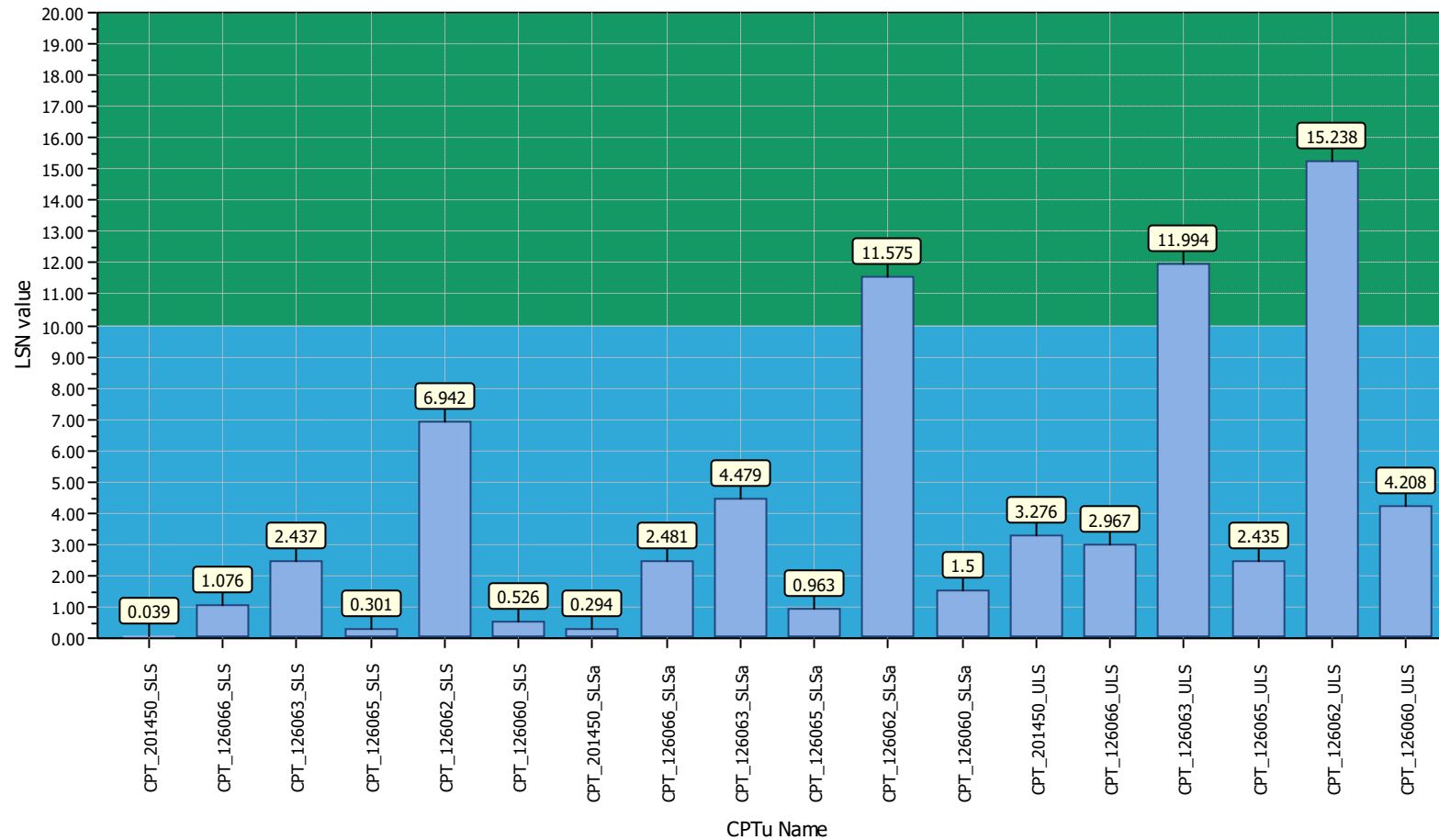
LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

Project title : CLiq Analysis

Location : 10 Bob Robertson Drive, Pegasus

Overall Liquefaction Severity Number report



LSN color scheme

- Severe damage
- Major expression of liquefaction
- Moderate to severe exp. of liquefaction
- Moderate expression of liquefaction
- Minor expression of liquefaction
- Little to no expression of liquefaction

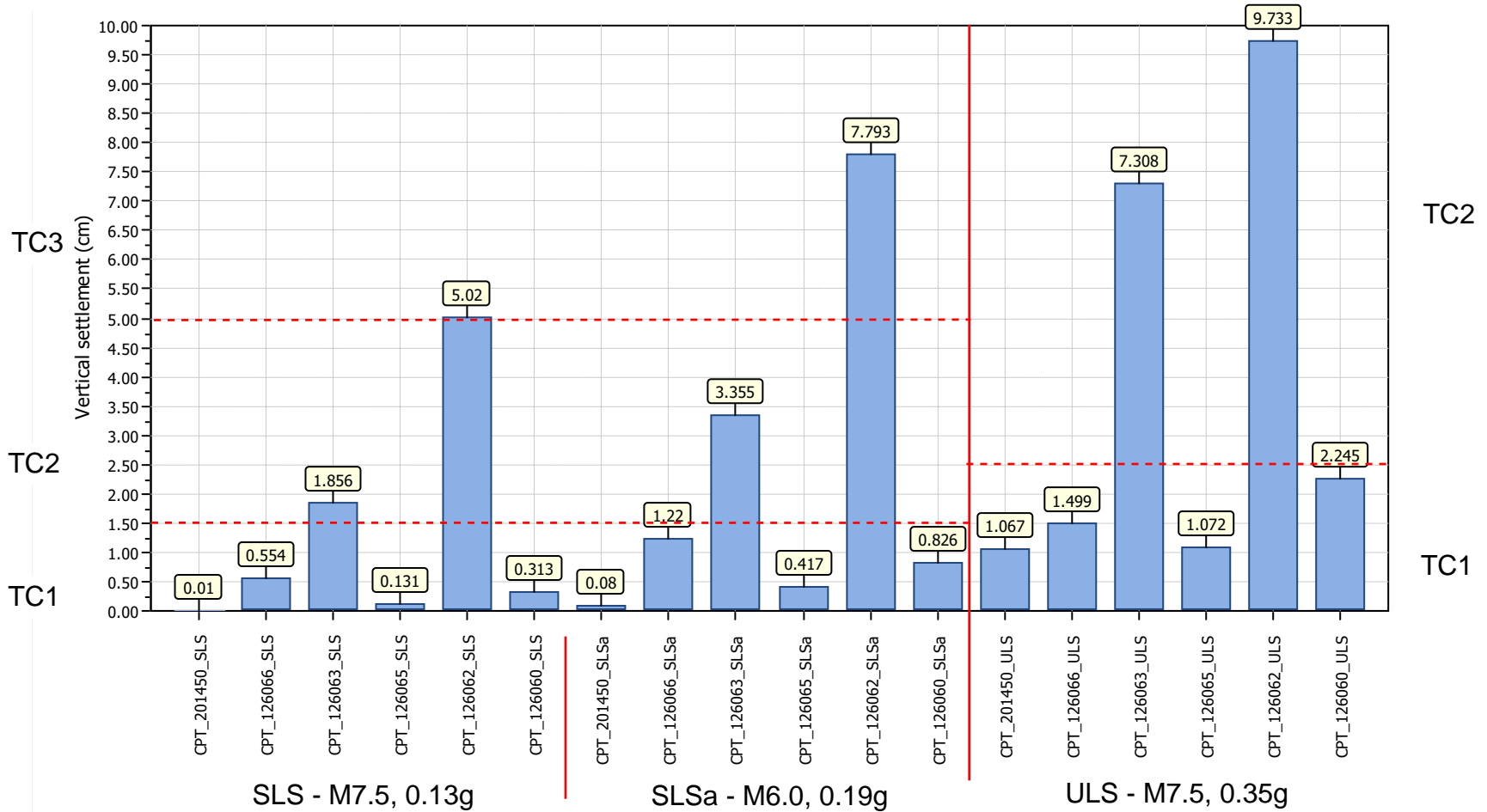
Basic statistics

Total CPT number: 18
 83% little liquefaction
 17% minor liquefaction
 0% moderate liquefaction
 0% moderate to major liquefaction
 0% major liquefaction
 0% severe liquefaction

Project title : CLiq Analysis

Location : 10 Bob Robertson Drive, Pegasus

Overall vertical settlements report



Appendix F. GNS Risk Assessment Method

Risk Assessment Method

Principles

The natural hazards risk assessment is based on GNS's risk-based land use planning toolbox¹². We have adapted the toolbox for the scale of the proposed residential subdivision and for the purposes of the RMA s106 reporting requirements. We have also considered Section 71 of the Building Act (2004), which includes a consent 'test' of whether land is likely to be subject to damage from a natural hazard.

The approach taken for this risk assessment considers whether land is likely to be subject to material damage from a natural hazard, where "likely" has been determined to be a 1 in 100-year return period event, rather than an extreme event with a low probability of occurrence. There are inevitable inconsistencies in the definitions used by different agencies and in this instance, we note that GNS refers to a 1 in 100-year return period event as a "possible" event.

The GNS toolbox report notes that many land use planning objectives, policies, rules, and decisions are based around a likelihood assessment of a natural hazard, such as the 1 in 100-year return period event. Where information exists that enables us to consider other relevant levels of likelihood, we have included additional commentary e.g., for different design earthquake scenarios.

Risk Calculation

In accordance with GNS's approach the risk is calculated as the product of the consequence and the likelihood, with the two inputs drawn from the Consequence Table (GNS Figure 3.4) and the Likelihood Scale (GNS Figure 3.5), as presented below.

Table 3. GNS Consequence Table

Severity of Impact	Built					
	Social / Cultural	Buildings	Critical Buildings	Lifelines	Economic	Health & Safety
Catastrophic (V)	≥25% of buildings of social/cultural significance within hazard zone have functionality compromised	≥50% of affected buildings within hazard zone have functionality compromised	≥25% of critical facilities within hazards zone have functionality compromised	Out of service for >1 month (affecting ≥20% of the town/city population) OR suburbs out of service for >6 months (affecting <20% of the town/city population)	>10% of regional GDP	>101 dead and/or >1001 injured
Major (IV)	11-24% of buildings of social/cultural significance within hazard zone have functionality compromised	21-49% of buildings within hazard zone have functionality compromised	11-24% of buildings within hazard zone have functionality compromised	Out of service for 1 week – 1 month (affecting ≥20% of the town/city population) OR suburbs out of service for 6 weeks to 6 months (affecting <20% of the town/city population)	1-9.99% of regional GDP	11 – 100 dead and/or 101-1001 injured

¹² <https://www.gns.cri.nz/Home/RBP/Risk-based-planning/A-toolbox>
Pegasus Makete, 1250 Main North Road, Woodend

Severity of Impact	Built					Health & Safety
	Social / Cultural	Buildings	Critical Buildings	Lifelines	Economic	
Moderate (III)	6-10% of buildings of social/cultural significance within hazard zone have functionality compromised	11-20% of buildings within hazard zone have functionality compromised	6-10% of buildings within hazard zone have functionality compromised	Out of service for 1 day to 1 week (affecting $\geq 20\%$ of the town/city population people) OR suburbs out of service for 1 week to 6 weeks (affecting $< 20\%$ of the town/city population)	0.1-0.99% of regional GDP	2 – 10 dead and/or 11 – 100 injured
Minor (II)	1-5% of buildings of social/cultural significance within hazard zone have functionality compromised	2-10% of buildings within hazard zone have functionality compromised	1-5% of buildings within hazard zone have functionality compromised	Out of service for 2 hours to 1 day (affecting $\geq 20\%$ of the town/city population) OR suburbs out of service for 1 day to 1 week (affecting $< 20\%$ of the town/city population)	0.01-0.09% of regional GDP	≤ 1 dead and/or 1 – 10 injured
Insignificant (I)	No buildings of social/cultural significance within hazard zone have functionality compromised	$< 1\%$ of affected buildings within hazard zone have functionality compromised	No damage within hazard zone, fully functional	Out of service for up to 2 hours (affecting $\geq 20\%$ of the town/city population) OR suburbs out of service for up to 1 day (affecting $< 20\%$ of the town/city population)	0.01% of regional GDP	No dead No injured

We have applied judgement and interpretation in the application of GNS's table to the scale and nature of the proposed subdivision.

For this assessment there are no social/cultural buildings to consider and the economic impacts are deemed not applicable. We use the 'Buildings' column as the basis to assess effects on the integrity of buildings (i.e., their amenity and life-safety function), and the 'Lifelines' column as the basis to assess the effects on utilities servicing the building/s.

Table 4. GNS Likelihood Scale

Level	Descriptor	Description	Indicative Frequency
5	Likely	The event has occurred several times in your lifetime	Up to once every 50 years
4	Possible	The event might occur in your lifetime	Once every 51 – 100 years
3	Unlikely	The event does occur somewhere from time to time	Once every 101 – 1000 years
2	Rare	Possible but not expected to occur except in exceptional circumstances	Once every 1001 – 2,500 years
1	Very rare	Possible but not expected to occur except in exceptional circumstances	2,501 years plus

To take a risk-based approach, the consequences and likelihood need to be quantified to provide a level of risk. To achieve this, a matrix is used that incorporates the relevant risk level, expressed as the consequences multiplied by likelihood (GNS Figure 3.8). The risk then ranges from 1 (extremely low) to 25 (extremely high).

Table 5. Color-coding the matrix based on level of risk

Likelihood	Consequences				
	1	2	3	4	5
5	5	10	15	20	25
4	4	8	12	16	20
3	3	6	9	12	15
2	2	4	6	8	10
1	1	2	3	4	5

GNS then suggests the consent status (treatment) of the activity can be presented as follows (GNS Figure 3.9):

Table 6. Level of risk and associated consent status

Level of risk	Consent
Acceptable	Permitted
Acceptable	Controlled
Tolerable	Restricted Discretionary
Tolerable	Discretionary
Intolerable	Non-complying, prohibited

GNS recommends that when assessing consequences, the final level of impact is assessed on the ‘first past the post’ principle, in that the consequence with the highest severity of impact applies.